Thermal buckling analysis of cross-ply plates based on new displacement field

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ABSTRACT

A higher-order displacement field is used for the analysis of the thermal buckling of composite plates subjected to thermal load; it is based on a constant "m", which is optimized to get results relatively close to those given by 3D elasticity theory. Adequate transverse shear strains distribution through the thickness and free stress surfaces of the plate is satisfied using this theory. Hamilton's principle is used to derive equations of motion, which are solved using Navier-type series for simply supported plates. Thermal buckling of cross-ply laminates with various (α_2/α_1) ratios, number of layers, aspect ratios, E_1/E_2 ratios, and stacking sequence for thick and thin plates is studied in detail. It is concluded that the obtained results using this displacement field are close to those calculated by 3D elasticity theory and other shear deformation plate theories when m=0.05.

Keywords: Critical temperature; Cross plates; Displacement field; Shear deformation theory; Thermal buckling.

INTRODUCTION

Thermal loads have an important role in several engineering industries, like aerospace and transportation structures, which widely used laminated composite materials; therefore, many researchers have studied thermal buckling of these materials based on different plates theories such as two-dimensional theory of elasticity (2D) (classical, first-order theory and higher-order theory) or three-dimensional theory of elasticity (3D), as provided by Noor et al. (1992).

M. Mansour Mohieddin Ghomshei and Amin Mahmoudi, (2010) implemented method of differential quadrature (DQM) to obtain the critical thermal buckling of the cross-ply thin plate; Le-Chung Shiau et al. (2010) used finite element as a solution method to investigate buckling temperature of angle and cross-ply plates; Mohamed Bourada et al. (2011) developed a new four-variable refined plate theory for thermal buckling analysis of functionally graded material (FGM) sandwich plates; Wael R. Abdul-Majeed et al. (2011) investigated thermal buckling of isotropic thermoelastic thin plates using classical theory and the Rayleigh-Ritz method; Adnan Naji Jameel et al. (2012) investigated buckling of laminates with different boundary conditions subjected to thermal and mechanical loads. They used different plate theories, classical laminated plate theory , higher-order shear deformation plate theory, and Finite element method; Marina Ćetković and Lénárt György, (2016) analyzed critical buckling temperature of angle-ply laminates using nine-node Lagrangian isoperimetric element to derive finite element model for the problem; M. Cetkovic, (2016) studied buckling temperature of composite plates, analytically by Navier's solution and numerically using the isoperimetric finite element approximation and based on Reddy Layerwise theory and its modified version; Youcef Beldjelili et al. (2016) used a four-variable refined plate theory to analyze hygro-thermo-mechanical bending of sigmoid functionally graded material plate.

R. Vescovini et al. (2017) used Ritz-based variable-kinematic formulation to study thermal buckling of composite plates and sandwich panels; Ait Atmane et al. (2017) used of an efficient beam theory that takes into account both shear deformation and thickness stretching effects by a parabolic variation for all displacements through the thickness to study bending, buckling, and free vibration of beams made from functionally graded material; Houdayfa Ounis and Mohamed-Ouejdi Belarbi (2017) studied the thermal buckling behavior of laminated plates with rectangular cut-outs using classical plate theory as a base for finite element method; Yufeng Xing and Zekun Wang (2017) concerned the critical buckling temperature of functionally graded thin plates based on classical plate theory; A. Menasria et al. (2017) proposed higher shear deformation theory, in which kinematics use only four variables and variation of transverse shear stress as a trigonometric with free plate surfaces reported, to obtain thermal buckling of plates; F. El-Haina et al. (2017) investigated the thermal buckling of thick plate sandwich functionally graded using sinusoidal shear deformation theory and stress function; Abdelbaki Chikh et al. (2017) presented a new displacement field for higher-order shear deformation theory, containing undetermined integral terms and four unknowns to study thermal buckling of composite plates; Bellifa et al. (2017) used a refined plate theory based on a new displacement field that has undetermined integral variables and contains only four unknowns to study buckling analysis of plates made from functionally graded; H.H. Abdelaziz et al. (2017) developed a simple hyperbolic shear deformation theory that has in-plane displacements field varying hyperbolically through the thickness, to investigate bending, vibration, and buckling of power graded material (PGM) sandwich plate with different boundary conditions.

Fouad Bourada et al. (2018) developed refined plate theory with four variables to analyze buckling of plates made from hybrid functionally graded; in this theory, variation of transverse shear deformation is a parabolic through thickness, satisfying surface conditions of the plate; Behrouz Karami et al. (2018) presented a new sizedependent quasi-3D plate theory, which has parabolic variations of displacements through the thickness, to analyze wave dispersion of functionally graded nanoplates resting on an elastic foundation and in hygrothermal environment; Amina Attia et al. (2018) used an efficient higher-order shear deformation theory contains hyperbolic distributions of the transverse shear strains to analyze thermomechanical bending of temperature-dependent functionally graded plates; A. Younsi et al. (2018) proposed two-dimensional and quasi-three-dimensional higher-order shear deformation theory for bending and free vibration investigation of functionally graded plates based on hyperbolic displacement function; Riad Hamza-Cherif et al. (2018) analyzed Euler-Bernoulli beam model with different boundary conditions to predict the vibration of a single-walled carbon nanotube embedded under thermal effect, using Differential Transform Method; A. Bouhadra et al. (2018) improved a higher shear deformation theory to investigate plates made from functionally graded, by accounting in-plane displacements that have undetermined integral terms and a parabolic variation for vertical displacement through the thickness; M. Benchohra et al. (2018) developed a quasi-3D sinusoidal shear deformation theory accounting for shear deformation and thickness-stretching influences using a trigonometric function for all displacements distribution along thickness, to analyze bending and free vibration for simply supported functionally graded plates; Moussa Abualnour et al. (2018) developed a new quasi-3D shear deformation theory including transverse shear strains parabolic variation to analyze free vibration of the simply supported functionally graded plates; H. Fourn et al. (2018) developed a high-order hyperbolic (HSDT) shear deformation theory based on a new field of displacement, which introduces indeterminate integral variables, which are only four variables, to analyze waves propagation of functionally graduated plates;

Djaloul Zarga et al. (2019) used a simple quasi-3D shear deformation theory based on a displacement field that has five variables and reported new kinematics that contain undetermined integral variables, for thermomechanical bending analysis of functionally graded material sandwich plates; Hadjira Hellal et al. (2019) developed a simple "four-variable shear deformation" plate model, which has a displacement field containing integral terms and transverse shear stress trigonometric variation to study the hygrothermal environment effects on buckling and dynamic of plates rest on elastic foundations; Zoulikha Boukhlif et al. (2019) investigated free vibration of functionally graded (FG) plates resting on elastic foundation using a simple quasi-3D higher shear deformation theory, which considered variation of displacement along thickness and has only four unknowns; Boulefrakh Laid et al. (2019) used quasi-3D hyperbolic shear deformation model for static and dynamic analyses of functionally graded plates resting on foundations with damping effect; Sabrina Boutaleb et al. (2019) studied free vibration analysis of the functionally graded rectangular nanoplates, using theory

of nonlocal elasticity based on the quasi-3D high shear deformation theory, and a cubic displacement function is used along the thickness; Zaoui et al. (2019) developed a two-dimensional and quasi-three-dimensional shear deformation theories that have a novel displacement field, which includes undetermined integral terms and contains fewer unknowns to model the free vibration of FG plates resting on elastic foundations.

Abdelkader Mahmoudi et al. (2019) developed a refined quasi-three-dimensional shear deformation theory that has a number of unknowns and governing equations, only four fewer than unknown displacement functions taken by other theories, to study thermomechanical response of functionally graded sandwich plates; Lynda Amel Chaabane et al. (2019) used a hyperbolic shear deformation theory to analyze static and dynamic behavior of simply supported FG-beam resting on Winkler–Pasternak foundation types; Fouad Bourada et al. (2019) investigated free vibration analysis of simply supported perfect and imperfect (porous) FG beams using a high-order trigonometric deformation theory that has three unknown and has a parabolic shear deformation variation along the thickness; Mohammed Medani et al. (2019) investigated static and dynamic behavior of plate made from Functionally Graded Carbon Nanotubes, reinforced porous sandwich (PMPV) polymer, and nanocomposite plate, using first-order shear deformation theory; Rafik Meksi et al. (2019) developed a new shear deformation plate theory based on a new displacement field containing integrals, which has four variables, and quasi-parabolic variation of transverse shear stress, to analyze the bending, buckling, and natural frequencies functionally graded material sandwich plates.

In the present work, critical temperature of simply supported laminated composite plate is obtained using high-order shear deformation theory of plate based on displacement field used by Mantari et al. (2011) for laminated plates static and free vibration analyses (they take m=0.5), which is optimized in the present work to be m=0.05, which gives critical temperature of laminated plates relatively close to that of the three-dimensional elasticity theory. The effects of many thin and thick plate parameters, such as aspect ratio, E_1/E_2 ratio, α_2/α_1 ratio, and different schemes for cross-ply, are investigated.

DISPLACEMENT FIELD

The new higher-order theory displacement field used by Mantari et al., 2011, is as follows:

$$u(x, y, z) = u_o(x, y) - z \left(\frac{\partial w}{\partial x}\right) + f(z)\theta_1(x, y)$$
(1)

$$v(x, y, z) = v_o(x, y) - z\left(\frac{\partial w}{\partial y}\right) + f(z)\theta_2(x, y)$$
⁽²⁾

$$w(x, y, z) = w_o(x, y) \tag{3}$$

where $u_o(x, y)$, $v_o(x, y)$, $w_o(x, y)$, $\theta_1(x, y)$, $\theta_1(x, y)$, $\theta_2(x, y)$ are displacement of plate middle surface, and f(z) represents modified function that give more accurate distribution for shear strain and stress in transverse planes of laminated plate. Assuming free boundary conditions of the plate at its top and bottom surfaces, the displacement field becomes:

$$u(x, y, z) = u_o(x, y) + z \left(\frac{m\pi}{h}\theta_1 - \frac{\partial w}{\partial x}\right) + \sin\frac{\pi z}{h}e^{m\cos\left(\frac{\pi z}{h}\right)}\theta_1$$
(4)

$$v(x, y, z) = v_o(x, y) + z \left(\frac{m\pi}{h}\theta_2 - \frac{\partial w}{\partial y}\right) + \sin\frac{\pi z}{h}e^{m\cos\left(\frac{\pi z}{h}\right)}\theta_2$$
(5)

$$w(x, y, z) = w_0 \tag{6}$$

where
$$f(z) = \sin \frac{\pi z}{h} e^{m \cos \left(\frac{\pi z}{h}\right)} + yz$$
, where $y = \frac{\pi m}{h}$, $m = \text{constant}$ (7)

The strain-displacement relations take the form, Reddy (2004).

$$\varepsilon_{xx} = \frac{\partial u}{\partial x}, \ \varepsilon_{yy} = \frac{\partial v}{\partial y}, \ \varepsilon_{xz} = \frac{\partial w}{\partial z}$$
 (8, 9 and 10)

$$\varepsilon_{xy} = \frac{1}{2} \left(\frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right) = \frac{1}{2} \gamma_{xy} , \ \varepsilon_{xz} = \frac{1}{2} \left(\frac{\partial u}{\partial z} + \frac{\partial w}{\partial x} \right) = \frac{1}{2} \gamma_{xz}$$
(11 and 12)

$$\varepsilon_{yz} = \frac{1}{2} \left(\frac{\partial v}{\partial z} + \frac{\partial w}{\partial y} \right) = \frac{1}{2} \gamma_{yz} \tag{13}$$

Strain as function of displacement field is obtained by substituting Equations (4, 5, 6) into Equations (8-13) and results in

$$\varepsilon_{xx} = \varepsilon_{xx}^{0} + z\varepsilon_{xx}^{1} + \sin\frac{\pi z}{h}e^{m\cos\left(\frac{\pi z}{h}\right)}\varepsilon_{xx}^{2}$$
(14)

$$\varepsilon_{yy} = \varepsilon_{yy}^{0} + z\varepsilon_{yy}^{1} + \sin\frac{\pi z}{h}e^{m\cos\left(\frac{\pi z}{h}\right)}\varepsilon_{yy}^{2}$$
(15)

$$\gamma_{xy} = \varepsilon_{xy}^{0} + z\varepsilon_{xy}^{1} + \sin\frac{\pi z}{h}e^{m\cos\left(\frac{\pi z}{h}\right)}\varepsilon_{xy}^{2}$$
(16)

$$\gamma_{xz} = \varepsilon_{xz}^{0} + (-m * \sin^2(\frac{\pi z}{h}) + \cos\frac{\pi z}{h}) \frac{\pi}{h} e^{m \cos(\frac{\pi z}{h})} \varepsilon_{xz}^{3}$$
(17)

$$\gamma_{yz} = \varepsilon_{yz}^{0} + \left(-m * \sin^2\left(\frac{\pi z}{h}\right)_+ \cos\frac{\pi z}{h}\right) \frac{\pi}{h} e^{m\cos\left(\frac{\pi z}{h}\right)} \varepsilon_{yz}^3$$
(18)

where

$$\begin{cases} \varepsilon_{xx}^{0} \\ \varepsilon_{yy}^{0} \\ \gamma_{xy}^{0} \end{cases} = \begin{cases} \frac{\partial u}{\partial x} \\ \frac{\partial u}{\partial x} \\ \frac{\partial u}{\partial x} \\ \frac{\partial u}{\partial y} + \frac{\partial v}{\partial y} \end{cases}, \begin{cases} \varepsilon_{xx}^{1} \\ \varepsilon_{yy}^{1} \\ \gamma_{xy}^{1} \end{cases} = \begin{cases} \frac{m\pi}{h} \frac{\partial \Theta_{1}}{\partial x_{1}} - \frac{\partial^{2}w}{\partial x^{2}} \\ \frac{m\pi}{h} \frac{\partial \Theta_{2}}{\partial y} - \frac{\partial^{2}w}{\partial y^{2}} \\ \frac{m\pi}{h} \frac{\partial \Theta_{2}}{\partial x_{1}} + \frac{m\pi}{h} \frac{\partial \Theta_{2}}{\partial y} \\ -2\frac{\partial^{2}w}{\partial x\partial y} \end{cases} \end{cases}, \begin{cases} \varepsilon_{xx}^{2} \\ \varepsilon_{yy}^{2} \\ \gamma_{xy}^{2} \end{cases} = \begin{cases} \frac{\partial \Theta_{1}}{\partial x} \\ \frac{\partial \Theta_{2}}{\partial y} \\ \frac{\partial \Theta_{2}}{\partial x} + \frac{\partial \Theta_{1}}{\partial y} \end{cases}$$
(19, 20 and 21)

$$\begin{cases} \gamma_{xz}^{0} \\ \gamma_{yz}^{0} \end{cases} = \begin{cases} m \frac{\pi}{h} \Theta_{1} \\ m \frac{\pi}{h} \Theta_{2} \end{cases}, \quad \begin{cases} \gamma_{xz}^{3} \\ \gamma_{yz}^{3} \end{cases} = \begin{cases} \Theta_{1} \\ \Theta_{2} \end{cases}$$
(22 and 23)

ENERGY PRINCIPLES

Principle of virtual displacements is used to derive equations of motion for thermal buckling of composite plate, Reddy (2004):

$$0 = \int_0^t \delta U + \delta V \tag{24}$$

where δU is expressed as

$$\delta U = \left[\int_{\frac{-h}{2}}^{\frac{h}{2}} \left\{\int_{Q}^{k} \sigma_{xx} \delta \varepsilon_{xx}^{k} + \sigma_{yy} \delta \varepsilon_{yy}^{k} + \sigma_{xy} \delta \varepsilon_{xy}^{k} + \sigma_{yz} \delta \varepsilon_{yz}^{k} + \sigma_{xz} \delta \varepsilon_{xz}^{k}\right] \partial x \partial y \left\} \partial z \right] = 0$$
(25)

$$\delta U = \int (N_1 \delta \varepsilon_{xx}^0 + M_1 \delta \varepsilon_{xx}^1 + P_1 \delta \varepsilon_{xx}^2 + N_2 \delta \varepsilon_{yy}^0 + M_2 \delta \varepsilon_{yy}^1 + P_2 \delta \varepsilon_{yy}^2 + N_6 \delta \varepsilon_{xy}^0 + M_6 \delta \varepsilon_{xy}^1 + P_6 \delta \varepsilon_{xy}^2 + Q_2 \delta \varepsilon_{yz}^0 + k_2 \delta \varepsilon_{xy}^3 + Q_1 \delta \varepsilon_{xz}^0 + k_1 \delta \varepsilon_{xz}^3 -) \partial x \partial y = 0$$
(26)

where N_i, Mi, Pi, Qi and K_i result from the following integration:

$$(N_{i}, M_{i}, P_{i}) = \sum_{k=1}^{N} \int_{z^{k-1}}^{z^{k}} \sigma_{i}^{k} \left(1, z, \sin\frac{\pi z}{h} e^{m\cos\left(\frac{\pi z}{h}\right)}\right) dz \qquad (i = 1, 2, 6)$$

$$(Q_1, K1) = \sum_{k=1}^{N} \int_{z^{k-1}}^{z^k} \sigma_5^k \left(1, \frac{\pi}{h} (-m * \sin^2(\frac{\pi z}{h}) + \cos\frac{\pi z}{h}) e^{m \cos\left(\frac{\pi z}{h}\right)} \right) dz$$

$$(Q_2, K2) = \sum_{k=1}^{N} \int_{z^{k-1}}^{z^k} \sigma_4^k \left(1, \frac{\pi}{h} (-m * \sin^2(\frac{\pi z}{h}) + \cos\frac{\pi z}{h}) e^{m \cos\left(\frac{\pi z}{h}\right)} \right) dz$$

substituting strains from Equations (14-23) into Equation (26) and integrating by parts, then we get

$$0 = -\int \left[\frac{\partial N_1}{\partial x} \,\delta u + \frac{m\pi}{h} \frac{\partial M_1}{\partial x} \,\delta \Theta_1 - \frac{\partial^2 M_1}{\partial x^2} \,\delta w + \frac{\partial P_1}{\partial x} \,\delta \Theta_1 + \frac{\partial N_2}{\partial y} \,\delta v + \frac{m\pi}{h} \frac{\partial M_2}{\partial y} \,\delta \Theta_2 - \frac{\partial^2 M_2}{\partial y^2} \,\delta w + \frac{\partial P_2}{\partial y^2} \,\delta \Theta_2 + \frac{\partial N_6}{\partial y} \,\delta \Theta_1 + \frac{m\pi}{h} \frac{\partial M_6}{\partial y} \,\delta \Theta_1 + \frac{m\pi}{h} \frac{\partial M_6}{\partial x} \,\delta \Theta_2 + 2 \frac{\partial^2 M_6}{\partial x \,\partial y} \,\delta w + \frac{\partial P_6}{\partial y} \,\delta \Theta_1 + \frac{\partial P_6}{\partial x} \,\delta \Theta_2 - \frac{m\pi}{h} Q_1 \,\delta \Theta_1 - \frac{m\pi}{h} Q_2 - K_1 \,\delta \Theta_1 - K_2 \,\delta \Theta_2 \right] \,\partial x \,\partial y \tag{27}$$

And virtual work done by thermal load δV is

$$\delta V = \int_{\Omega} \left\{ N_x^T \,\delta \left(\frac{\partial w}{\partial x} \right)^2 + N_y^T \,\delta \left(\frac{\partial w}{\partial y} \right)^2 + N_{xy}^T \,\delta \left(\frac{\partial w}{\partial x} \right) \times \left(\frac{\partial w}{\partial y} \right) \right\} dxdy \tag{28}$$

EQUATIONS OF MOTION

Substituting Equations (27–28) into Equation (24), also equating coefficients of (δu , δv , δw , $\delta \Theta_1$, $\delta \Theta_2$) over Ω_0 of Equation (24) to zero separately will result the following equations of motion:

$$\delta u: \frac{\partial N_x}{\partial x} + \frac{\partial N_{xy}}{\partial y} = 0$$
⁽²⁹⁾

$$\delta v: \frac{\partial N_y}{\partial y} + \frac{\partial N_{xy}}{\partial x} = 0$$
(30)

$$\delta_{W}: \frac{\partial^2 M_x}{\partial x^2} + \frac{\partial^2 M_y}{\partial y^2} + 2 \frac{\partial^2 M_{xy}}{\partial x \partial y} + N_x^T \left(\frac{\partial^2 w}{\partial x^2} \right) + N_y^T \left(\frac{\partial^2 w}{\partial y^2} \right) = 0$$
(31)

$$\delta\Theta_{1:} \frac{m\pi}{h} \frac{\partial M_x}{\partial x} + \frac{m\pi}{h} \frac{\partial M_{xy}}{\partial y} + \frac{\partial P_x}{\partial x} + \frac{\partial P_{xy}}{\partial y} - \frac{m\pi}{h} Q_x - K_x = 0$$
(32)

$$\delta\Theta_2: \frac{m\pi}{h} \frac{\partial M_y}{\partial y} + \frac{m\pi}{h} \frac{\partial M_{xy}}{\partial x} + \frac{\partial P_y}{\partial y} + \frac{\partial P_{xy}}{\partial x} - \frac{m\pi}{h} Q_y - K_y = 0$$
(33)

The two-dimensional plate reduced stiffness Q_{ij} is $Q_{11} = \frac{E_1}{1 - v_{12}v_{21}}$, $Q_{12} = \frac{v_{12}E_2}{1 - v_{12}v_{21}}$, $Q_{11} = \frac{E_2}{1 - v_{12}v_{21}}$, $Q_{66} = G_{12}$, $Q_{44} = G_{23}, Q_{55} = G_{13}$ (34)

where E_1 and E_2 = Young's modulus in directions 1 and 2 of composite plate, while G_{12} , G_{23} and G_{13} = shear modulus of plate in three orthogonal planes. Also, v_{12} and v_{21} are poison's ratio in plane 1-2. Orthotropic lamina stress

components are related to strain as follows:

$$\begin{cases} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{cases} = \begin{bmatrix} Q_{11}Q_{12}Q_{16} \\ Q_{12}Q_{22}Q_{26} \\ Q_{16}Q_{26}Q_{66} \end{bmatrix} \begin{cases} \varepsilon_{xx} - \alpha_{xx}\Delta T \\ \varepsilon_{yy} - \alpha_{xx}\Delta T \\ \gamma_{xy-}2\alpha_{xy}\Delta T \end{cases}, \begin{cases} \sigma_{yz} \\ \sigma_{xz} \end{cases} = \begin{bmatrix} Q_{44} & Q_{45} \\ Q_{45} & Q_{55} \end{bmatrix} \begin{cases} \gamma_{yz} \\ \gamma_{xz} \end{cases}$$
(35)

Also resulting forces are related to the strains as follows:

$$\begin{pmatrix} N_{x} \\ N_{y} \\ N_{xy} \\ M_{xy} \\ M_{y} \\ M_{y} \\ M_{xy} \\ P_{x} \\ P_{y} \\ P_{xy} \end{pmatrix} = \begin{bmatrix} A_{11}A_{12}A_{16} B_{11}B_{12}B_{16} E_{11}E_{12}E_{16} \\ A_{12}A_{22}A_{26} B_{12}B_{22}B_{26} E_{12}E_{22}E_{26} \\ A_{16}A_{26}A_{66} B_{16}B_{26}B_{66} E_{16}E_{26}E_{66} \\ B_{11}B_{12}B_{16} D_{11}D_{12}D_{16} F_{11}F_{12}F_{16} \\ B_{12}B_{22}B_{26} D_{12}D_{22}D_{26} F_{12}F_{22}F_{26} \\ B_{16}B_{26}B_{66} D_{16}D_{26}D_{66} F_{16}F_{26}F_{66} \\ E_{11}E_{12}E_{16} F_{11}F_{12}F_{16} H_{11}H_{12}H_{16} \\ E_{12}E_{22}E_{26} F_{12}F_{22}F_{26} H_{12}H_{22}H_{26} \\ E_{16}E_{26}E_{66} F_{16}F_{26}F_{66} H_{16}H_{26}H_{66} \end{bmatrix} \begin{cases} \epsilon_{y}^{0} \\ \epsilon_{y}^{0} \\ \epsilon_{y}^{0} \\ \epsilon_{xy}^{0} \\ \epsilon_{xy}^{1} \\ \epsilon_{y}^{1} \\ \epsilon_{y}^{2} \\ \epsilon_{xy}^{2} \\ \epsilon_{xy}^{$$

$$\begin{cases} Q_{x} \\ Q_{y} \\ K_{x} \\ K_{y} \end{cases} = \begin{bmatrix} A_{44} & A_{45} J_{44} & J_{45} \\ A_{45} & A_{55} J_{45} & J_{55} \\ J_{44} & J_{45} L_{44} & L_{45} \\ J_{45} & J_{55} L_{45} & L_{55} \end{bmatrix} \begin{cases} \gamma_{yz}^{0} \\ \gamma_{xz}^{0} \\ \gamma_{yz}^{3} \\ \gamma_{xz}^{3} \\ \gamma_{xz}^{3} \end{cases}$$
(37)

while in plane thermal loads are

$$\begin{cases} N_x^T \\ N_y^T \\ N_{xy}^T \\ N_{xy}^T \end{cases} = \sum_{k=1}^N \int_{z^k}^{z^{k+1}} \begin{bmatrix} Q_{11} & Q_{12} & Q_{16} \\ Q_{12} & Q_{22} & Q_{26} \\ Q_{16} & Q_{26} & Q_{66} \end{bmatrix} \begin{cases} \alpha_{xx} \\ \alpha_{yy} \\ 2\alpha_{xy} \end{cases} \Delta T dz$$
(38)

where
$$A_{ij} = \int_{\frac{-h}{2}}^{\frac{h}{2}} Q_{ij} dz$$
 $i = (1,2,4,5,6)$ (39)

$$(B_{ij}, D_{ij}, E_{ij}) = \int_{\frac{-h}{2}}^{\frac{h}{2}} Q_{ij}(z, z^2, \sin(\frac{\pi z}{h})e^{m\cos(\frac{\pi z}{h})}, \quad (i, j=1, 2, 6)$$
(40)

$$(F_{ij}, H_{ij}) = \int_{\frac{-h}{2}}^{\frac{h}{2}} Q_{ij} \left(\sin\left(\frac{\pi z}{h}\right) e^{m \cos\left(\frac{\pi z}{h}\right)} * z, \sin^2\left(\frac{\pi z}{h}\right) e^{2m \cos\left(\frac{\pi z}{h}\right)} \right) \qquad (i, j=1, 2, 6)$$
(41)

$$J_{ij} = \int_{\frac{-h}{2}}^{\frac{h}{2}} Q_{ij} \frac{\pi}{h} e^{m \cos\left(\frac{\pi z}{h}\right)} (-m * \sin^2\left(\frac{\pi z}{h}\right) + \cos\frac{\pi z}{h}) dz$$
(42)

$$L_{ij} = \int_{\frac{-h}{2}}^{\frac{h}{2}} Q_{ij} \left(\frac{\pi}{h}\right)^2 e^{2m\cos\left(\frac{\pi z}{h}\right)} \left(-m * \sin^2\left(\frac{\pi z}{h}\right) + \cos\frac{\pi z}{h}\right)^2 dz \quad i, j = (4,5)$$
(43)

And, α_{xx} , α_{yy} , α_{xy} are coefficients of thermal expansion for composite plate.

NAVIER'S SOLUTION

Simply supported boundary conditions for cross-ply plate are satisfied by Navier's generalized displacements, which are trigonometric series in terms of unknown amplitudes as shown below Reddy (2004):

$$u(x, y, t) = \sum_{M=1}^{\infty} \sum_{N=1}^{\infty} U_{MN} \cos(\alpha x) \sin(\beta y)$$
(44)

$$\mathbf{v}(\mathbf{x}, \mathbf{y}, \mathbf{t}) = \sum_{M=1}^{\infty} \sum_{N=1}^{\infty} V_{MN} \sin(\alpha \mathbf{x}) \cos(\beta \mathbf{y})$$
(45)

$$w(x, y, t) = \sum_{M=1}^{\infty} \sum_{N=1}^{\infty} W_{MN} \sin(\alpha x) \sin(\beta y)$$
(46)

$$\theta_1(\mathbf{x}, \mathbf{y}, \mathbf{t}) = \sum_{M=1}^{\infty} \sum_{N=1}^{\infty} \theta_{1_{MN}} \cos(\alpha \mathbf{x}) \sin(\beta \mathbf{y})$$
(47)

$$\theta_2(\mathbf{x}, \mathbf{y}, \mathbf{t}) = \sum_{M=1}^{\infty} \sum_{N=1}^{\infty} \theta_{2_{MN}} \sin(\alpha \mathbf{x}) \cos(\beta \mathbf{y})$$
(48)

And: $\alpha = \frac{M\pi}{h}$, $\beta = \frac{N\pi}{h}$, $(U_{MN}, V_{MN}, W_{MN}, \theta_{1_{MN}}, \theta_{2_{MN}})$, are unknown amplitudes.

EIGNVALUE PROBLEM

Express equations of motion (29-33) in terms of displacements by using Equations (36 - 43) and then solved using Equations (44-48), and the following eigenvalue problem is developed:

$$\begin{pmatrix} \begin{bmatrix} c_{11} & c_{12} & c_{13} & c_{14} & c_{15} \\ & c_{22} & c_{23} & c_{24} & c_{25} \\ & & c_{33-\alpha^2 N_x^T - \beta^2 N_y^T - \alpha \beta N_{xy}^T} & c_{34} & c_{35} \\ & & & c_{44} & c_{45} \\ & & & & c_{55} \end{bmatrix} \end{pmatrix} \{d\} = 0$$

$$(49)$$

where $\left\{ {{d_{ij}}} \right\} = \left\{ {{U_{mn}},{V_{mn}},{W_{mn}},\,{\theta _{1}}_{mn},{\theta _{2}}_{mn}} \right\}$

And C_{ij} is the element of stiffness, from its determinant thermal buckling is obtained.

RESULTS AND DISCUSSION

Using the above analytical solutions of the HOSDT based on displacement function given by J.L. Mantari et al. (2011), a computer program is built using MATLAB (R2015a). Design parameters effect such as thickness ration (a/h), aspect ratio (a/b), orthotropy ratio E_1/E_2 and coefficient of expansion ratio (α_2/α_1) on thermal buckling of orthotropic plates are investigated. Suggested above solution is verified by comparing present results with those given by other researchers and give good agreement as shown in Table (1), for different number of symmetric cross-ply and different thickness ratio (a/h), and for antisymmetric cross-ply plate and with 3-dimensional solution proposed by Noor et al., 1992 as listed in Table (2).

a/h	Defense	Tcr				
	Kelerences	4layers	8layers	20layers		
	Present	0.0533	0.03113	.02478		
4	L. X. SUN and T. R. HSIJ, 1990	0.0575	0.0575 0.0315			
	Shu Xiaopingt and Sun Liangxint, 1994	0.0554	0.0279	0.0218		
	Present	0.14199	0.07737	0.06094		
10	L. X. SUN and T. R. HSIJ, 1990	0.1522	0.0797	.0621		
	Shu Xiaopingt and Sun Liangxint, 1994	0.1436	0.0747	0.0584		
100	Present	0.2364	0.11255	0.0856		
	L. X. SUN and T. R. HSIJ, 1990	0.2435	0.1148	0.0872		
	Shu Xiaopingt and Sun Liangxint, 1994	0.2431	0.1147	0.0817		

Table 1. Normalized critical temperature $(T^*cr = T^*a^{2*}h/\pi^{2*}D_{22})$ for symmetric cross-ply square plate,
mode (M=1, N=1).

Table 2. Normalized critical temperature (Tcr = T*1e-6) for cross-ply plate (a/b=1), $E_1/E_2=15$, $G_{12}=G_{13}=.5E_2$, $G_{23}=.3356E_2$, $v_{12}=0.3$, $\alpha_2/\alpha_1=0.015$.

a/h	(0/90) mode (M=1, N=1)		[(mode (M	Discrepancy % (present and Noor		
	References	Tcr	References	Ter	et al. (1992))	
4	Present	0.1972	Present	0.18717	5.05	
4	M. Cetkovic, 2016	0.1834	Noor et al. 1992	0.1777	5.05	
10	Present	0.0457	Present	0.05923	2.2	
	M. Cetkovic, 2016	0.4397 *10-1	Noor et al. 1992	0.5782 x 10 ⁻¹	2.3	
20	Present	0.0122	Present	0.01755	0.011	
20	M. Cetkovic, 2016	0.1184 *10-1	Noor et al. 1992	0.1739 x 10 ⁻¹	0.911	
100	Present	0.0005007	Present	0.0007479	0.010	
	M. Cetkovic, 2016	0.4857 *10-3	Noor et al. 1992	0.7463 x10 ⁻³	0.213	

Changing thermal expansion coefficient ratio (α_2 / α_1) effect on critical buckling temperature of 8layer thick and thin plates (with different (a/h) ratio), are listed in Table (3) and shown in Fig.(1), as expected critical temperature decrease when (α_2 / α_1) increase and (a/H) increase since stiffness decrease when plate become thinner.

a/h	(α_2 / α_1)	T*cr
	2	0.03268
	4	0.02973
4	6	0.02726
	8	0.02517
	10	0.02338
	2	0.08174
	4	0.07435
10	6	0.06819
	8	0.06297
	10	0.05849
	2	0.11814
	4	0.10746
100	6	0.09856
	8	0.09101
	10	0.08454

Table 3. Effect of (α_2/α_1) on normalized critical temperature $(T^*cr = T^*a^{2*}h/\pi^{2*}D_{22})$ for symmetric cross-ply square
plate, mode (M=1, N=1), No. of layers=8.



Figure 1. Effect of (α_2/α_1) on normalized critical temperature $(T^*cr = T^*a^{2*}h/\pi^{2*}D_{22})$ for symmetric cross-ply square plate, mode (M=2, N=1), No. of layers=8.

Aspect ratio effect on critical thermal buckling of 4layer thick and thin composite plates, are listed in Table (4), normalized critical temperature increase as aspect ratio increase because decreasing stiffness (D_{22}) or critical temperature decrease as (a/b) increase, also critical temperature for antisymmetric cross-ply is larger than that for symmetric since stiffness is better for the former. Different critical thermal buckling modes for plates with different aspect ratio are shown in Figs. (2-5).

		Symmetric layers	5	Antisymmetric layers			
a/b		a/h		a/h			
	4	10	100	4	10	100	
1	0.05338	0.14199	0.23647	0.01974	0.04642	0.06298	
2	0.06925	0.19843	0.32143	0.02613	0.09148	0.20054	
3	0.08183	0.29227	0.61414	0.03170	0.12690	0.44651 (M=2, N=1)	
4	0.09136	0.36841	1.05542	0.03714 (M=2, N=1)	0.14713 (M=2, N=1)	0.77310 (M=2, N=1)	

Table 4. Effect of (a/b) on normalized critical temperature ($T^*cr = T^*a^{2*}h/\pi^{2*}D_{22}$) for cross-ply square plate, mode(M=1, N=1), No. of layers=4.



Figure 2. Thermal Buckling mode for symmetric cross-ply square plate, mode (M=1, N=1), No. of layers=8, a/h=4, a/b=1.



Figure 3. Thermal Buckling mode for symmetric cross-ply square plate, mode (M=2, N=1), No. of layers=8, a/h=4, a/b=2.



Figure 4. Thermal Buckling mode for symmetric cross-ply square plate, mode (M=3, N=1), No. of layers=8, a/h=4, a/b=3.



Figure 5. Thermal Buckling mode for symmetric cross-ply square plate, mode (M=4, N=1), No. of layers=8, a/h=4, a/b=4.

Tables 5, 6, and 7 show effect of changing (E_1/E_2) on critical temperature for 4, 8, and 20 layers symmetric and antisymmetric cross-ply plates, since stiffness increase when increasing orthotropic ratio therefore normalized critical temperature decrease $(D_{22} \text{ increase})$. Material used for all present work results are as follows: $E_1/E_2=25$, $G_{12}=G_{13}=.5E_2$, $G_{23}=.2E_2$, $v_{12}=0.25$, $\alpha_2/\alpha_1=3$, except when mentioned.

Table 5. Effect of (E_1/E_2) on normalized critical temperature $(T^*cr = T^*a^{2*}h/\pi^{2*}D_{22})$ for symmetric cross-ply squareplate, mode (M=1, N=1), No. of layers=4.

	Symmetric layers			Antisymmetric layers			
a/H	E_1/E_2			E ₁ / E ₂			
	10	25	40	10	25	40	
4	0.16029	0.0533	0.02704	0.07105	0.01974	0.009339	
10	0.32783	0.14199	0.08099	0.12715	0.04642	0.02591	
100	0.42475	0.2364	0.16295	0.14983	0.06298	0.03974	

Table 6. Effect of (E_1/E_2) on normalized critical temperature $(T^*cr = T^*a^{2*}h/\pi^{2*}D_{22})$ for symmetric cross-ply square plate, mode (M=1, N=1), No. of layers=8.

	Symmetric layers			Antisymmetric layers			
a/H	E ₁ /E ₂			E ₁ /E ₂			
	10	25	40	10	25	40	
4	0.10535	0.03113	0.01503	0.07491	0.021072	0.010034	
10	0.19832	0.07787	0.04386	0.13706	0.051216	0.02864	
20	0.22842	0.101379	0.062813	0.155864	0.064974	0.039749	
100	0.24022	0.11255	0.07337	0.16304	0.071124	0.043543	

Table 7. Effect of (E_1/E_2) on normalized critical temperature $(T^*cr = T^*a^{2*}h/\pi^{2*}D_{22})$ for symmetric cross-ply square plate, mode (M=1, N=1), No. of layers=20.

	Symmetric layers			Antisymmetric layers			
a/H	E ₁ /E ₂			E ₁ /E ₂			
	10	25	40	10	25	40	
4	0.08628	0.02478	0.01187	0.07604	0.02146	0.010239	
10	0.159375	0.06094	0.03422	0.13984	0.05255	0.029412	
20	0.181889	0.077915	0.04797	0.15929	0.066935	0.041027	
100	0.190541	0.0856	0.05517	0.16673	0.07340	0.047027	

CONCLUSIONS

Thermal buckling solution of thick and thin cross-ply composite plates is developed using new displacement function suggested by Mantari et al. for static and free vibration analyses of composite plate (they used m=0.5), but "m" is changed in present work to 'm=.05', at which it gives results that agree well with those of the 3D elasticity theory with maximum discrepancy (5.05%) and other plate shear theories. As expected critical temperature is decreased

as thickness ratio, thermal coefficient ratio α_2/α_1 and aspect ratio increased, while the buckling temperature increases when E_1/E_2 increases and is larger for thick than thin plates, and also antisymmetric cross-ply will buckle with a temperature larger than those for symmetric ply.

Mode number of thermal buckling for simply supported cross-ply plate does not change when using this new displacement field.

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