

**A CIRCULAR ECONOMY IN WASTE MANAGEMENT CARRYING OUT
EXPERIMENTAL EVALUATION OF COMPRESSED STABILIZED EARTH BLOCK
USING MUNICIPAL SOLID WASTE INCINERATOR FLY ASH**

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ABSTRACT

The proper reuse and treatment Municipal solid waste incinerator fly ash is a is current a global concern. MSWI fly ashes possess a high concentration of SiO₂, allowing them to be utilized as a raw material in the production of CSEB. This research looks into compressed stabilized earth blocks (CSEBs) that use municipal solid waste incinerator fly ash (MSWIFA) as an alternative to soil-sand mixture and sand. The experiment was divided into two phases: in the first, the effect of municipal solid waste incinerator fly ash on substituting soil-sand mix without affecting original performance, as well as resistance to sulfate attack, was emphasized. The effect of MSWIFA particle size and replacement ratio on replacing natural sand was then investigated. The analysis reveals that including MSWIFA into a soil-sand mixture considerably improved block performance, particularly under wetting–drying cycles and sulfate attack. MSWIFA particle size and replacement ratio have a significant influence on block strength and water absorption. Compressive and flexural strength are improved by the addition of 20% MSWIFA with a particle size of 0/4.75 mm. As a result, the research establishes a new investigation into the environmental recycling of MSWIFA in the context of the circular economy.

Keywords: MSWIFA, CSEB, sulfate attack, compressive strength and circular economy.

INTRODUCTION

Earthen materials are mainly used in developing countries because of their cost efficiency, earthen materials also perform better in terms of strength and durability. Due to its multiple advantages, Earth has been used as a building material and is widely available at less cost[1]. Earthen materials are also an eco-friendly alternative to energy-intensive building materials that improve buildings' carbon footprint. The several buildings made from earth-

materials are old, yet still solid and rigid[2]. Furthermore, when using land as a construction material the need for skilled labour is not required. Bricks today are seen as one of the main building material [3] [4]. Innovative new building materials that can be locally made and easily constructed will help reduce costs for the housing sector, especially with the lower incoming segments [5]. Stabilized earth appears to be notable among sustainable building techniques. Compressed stabilized earth blocks (CSEBs) have been employed in several countries throughout the past 50 years. CSEBs are produced by compressing a soil moist mixture and the appropriate stabilizer in a high density block in a manually operated press[6]. These CSEBs are 2.5 times larger than normal burned clay bricks and consequently less joint building is faster. Utilization of industrial wastes in construction activities provide social, economic and environmental benefits[7]. Materials such as Fired Clay Brick (FCB)[8], concrete blocks became more and more common around the globe. Consequently, eco-friendly building materials for sustainable development are an essential issue that once again disputes the usage of raw earth. Recently, extensive study has been done to make land a sustainable building material. This has led to the development of Compressed Stabilized Earth Block (CSEB)[9]. Compressed Earth Blocks (CEBs) consist of the high-pressure compression of the soil. In order to prepare the mix for CEB, stabilizers are next added in particular quantities and called CSEB.

A great amount of municipal waste is produced worldwide and its treatment in most countries and regions becomes urgent. The traditional method of disposal is sanitary landfills, which consume land resources while also posing a potential threat to underground water and soil. As a result of the volume reduction and energy recovery from municipal solid waste, incineration is becoming more popular around the world. Approximately 25% of the entire mass of MSW after incineration is MSWIFA, which must be disposed of in landfills [10]. Recent developments indicate that future advances in resource efficiency are achieved with in a circular economy, trash becomes a resource and completing the loop. The management of solid waste is a significant responsibility globally; our country (India) buries a mound of garbage. The country produces more than 1.50 lakh metric tons of solid waste every day, based on the CPHEEO report. In Chennai, every day, around 5400 MT of garbage waste and 700 MT of construction and Demolition waste is collected. The waste generation by category residential 68%, Commercial 16%, School and Institution 14%, industrial and hospital. The garbage waste is segregated into biodegradable 40% and non-biodegradable 60%. By burning per ton of non-biodegradable waste, 20% of ash generated. To reuse solid waste will preserve

the future environment. The MSWIFA may be a better alternative for construction material. The intention was thus to contribute in the production industries to a circular economy[11].

This research focused on achieving nearly zero-energy buildings by using municipality solid waste. In India, municipality solid waste is dumped in landfills[12]. Reusing municipality solid waste may preserve the environment[13]. Many countries have undertaken recycling programs to assure the MSWIFA, such as aggregates, masonry blocks, highways and as an alternative to natural resources in building[14]. Using MSWIFA as an alternative construction material will improve its energy efficiency and compressive strength. The MSWIFA used as the construction material will increase the thermal comfort of the buildings. As a result of these issues, the authors decided to include MSWIFA in their CSEB preparation. Due to the soil demand, MSWIFA is the best option to replace the soil in manufacturing the CSEB and its also cost efficient [1]. Significant improvements have been seen recently in the use of such waste materials and usage rates vary in different countries from 27 to 90%. It is important to note that the proper use of this trash remains a serious challenge in emerging and impoverished countries. For example, in India, the utilization rate is the lowest (27%). Therefore, the incorporation of MSWIFA in CSEB might be a viable approach, which not only recycling MSWIFA as raw material and saving clay resource. The socioeconomic context of their native regions has a significant impact on the features of MSWIFA[10]. Therefore, the results of the present investigation may be used to conduct comparative studies of not only science but also socioeconomic interest[15]. The objective in this research was to assess the possible use of MSWIFA for the fabrication of CSEB as a raw material. As a result, we intend to contribute to waste management in a circular economy.

MATERIAL AND METHODS

Soil

The utilised soil has been gathered from 0.5–1.5 m under land level in Auroville, Pondicherry. The Atterberg limits, shrinking limit, free swell index, and conventional proctor compaction tests were used to determine the distribution of soil suitability. Table 1 outlines the index characteristics of the soil with liquid and plastic limitations of 51.78% and 22.67%. It shows that the soil is quite fine with a 97.12% fraction of clay and silt. The soil activity is 0.69 which usually indicates active in accordance with mineralogical data. Depending on soil type, the optimum clay content for CEB soils varies between 8.65% and 22.0%. The distribution of soil in grain size falls outside of the recommended IS 1725 envelope for producing compressed earth blocks. In addition to this, IS 1725 recommended a 30–75% and

50–80% CEB sand concentration. Mixtures with various percentages of sand and soil were therefore made to get the best mixing ratio. A soil-sand ratio mix 30:70 has been selected to achieve maximum dry density, which corresponds to previous studies. The highly fine, grained CSEB earth has a maximum density and compressive strength of 50-60%.

Table 1. Soil and Reconstitution Soil properties

Properties	Soil	Reconstitutes soil
Grain Size Distribution		
Silt	61.45%	20.45%
Clay	35.67%	10.67%
Sand	2.88%	61.88%
Atterberg Limit		
Liquid Limit	51.78%	25.7%
Plastic Limit	25.55%	20.8%
Plasticity Index	22.67%	8.2%
Proctor test		
Optimum moisture content	19.67%	21.45%
Maximum dry density	1820kg/m ³	1923 kg/m ³
Shrinkage limit	18.23%	-
IS Soil classification	CH	SM
Activity	0.69	-

Municipal solid waste to Municipal solid waste incinerator fly ash

Every day around 5400 MT of garbage is collected from the Chennai city. Source separation is promoted to reduce the waste coming to the Landfill. The source separated non-biodegradable (Dry Waste) are dumped in two dumping station. Incineration has overtaken landfilling as the most important option for disposal of the increasing volumes of municipal solid waste (MSW) generated in Chennai. The non-biodegradable MSWI fly ash has high silica, therefore it is suitable for replacement of construction material. In Chennai, there are two Incinerator plant at manali and kodungaiyur north madras. Each plant has capacity of 200T respectively. In 50T plant, production of MSWI fly ash is 2000kg per day in each plant. Fig.1. (a) & (b) MSWI plant, (c) MSWIFA



Figure.1. (a) & (b) Municipal solid waste incinerator plant, (c) MSWIFA

Sand and Municipal solid waste incinerator fly ash

Municipal solid waste incinerator fly ash collected from local incinerator plant and natural river sand was used. The physical properties of river sand and MSWIFA are presented in Table 2. The chemical properties of MSWIFA are shown in Tables 3.

Table 2. Physical properties of sand and MSWIFA. **Table 3** Chemical properties of MSWIFA

S.No	Properties	River sand	MSWIFA
1	Water absorption	1.07%	14.08%
2	Specific gravity	2.7	2.4
3	Fineness modulus	1.7	2.3
4	Loose bulk density	1400kg/m ³	1105kg/m ³
5	Compacted bulk density	1612kg/m ³	1356kg/m ³

Components	MSWIFA
SiO ₂	64.75
Al ₂ O ₃	0.78
Fe ₂ O ₃	0.38
CaCO ₃	14.85
Mg O	0.74
C	15.53
Fe ₂ O ₃	-

The Fig.2. Shows the SEM analysis of MSWIFA in 6500 magnifications which shows the crystalline nature of the structure similar to the SiO₂ raw material in the sand. The indicates there is some pores in the material due to the uneven nature of the material. The microstructure of 12000 magnification shows the particles are much dense it is similar to the sand. The MSWIFA particles distributed equally throughout the matrix, forming a strong bond with the soil–cement matrix.

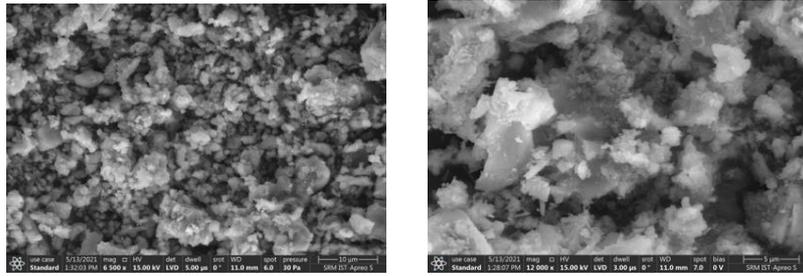


Figure.2. SEM image of MSWIFA

Cement

The cement used is standard Portland cement of grade 43, which conforms to IS 8112. Because larger soil contents are uneconomical, all mixtures are cement stabilized to a limit of 10% by soil weight.

Mixture Proportion of CSEB

A CSEB were prepared in different mix ratio and categorized in phase I and phase II. In phase I, MSWIFA was replaced by 7%, 14%, 21% and 28% of the soil-sand combination. Due to the low clay content of the soil, the addition of MSWIFA was limited to 28%. After this percentage, the block begins to crumble. It can be explained that, immediately after pressing, soil containing low clay levels may have problems handling blocks, because of a lack of initial cohesion[16]. In phase II, sand was replaced by MSWIFA. Tables 4, 5 lists the material proportions of the mixtures for both phase I and phase II.

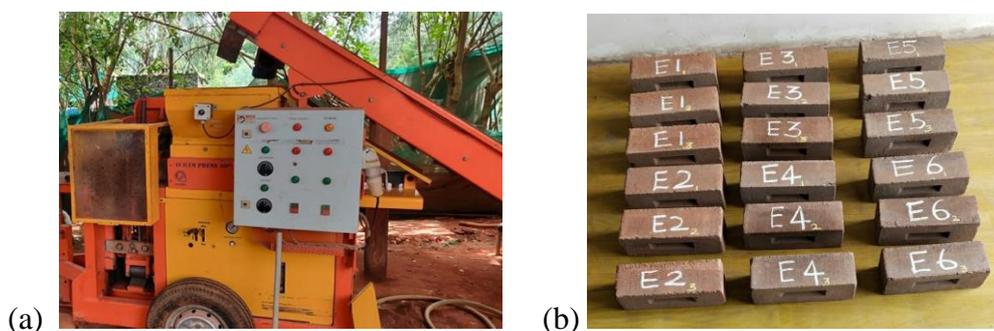
Sample	MSWIFA (%)	Soil-Sand (%)	Cement (%)

Sample	Cement (%)	Soil (%)	MSWIFA (%)	Sand (%)
E1	10	30	0	70
E2	10	30	14	56
E3	10	30	28	42
E4	10	30	42	28
E5	10	30	56	14
E6	10	30	70	0

IF	0	100	10
IF7	7	94	10
IF14	14	88	10
IF21	21	82	10
IF28	28	76	10

Table 4. Phase I – Mix proportions**Table 5.** Phase II – Mix proportions**CSEB production**

CSEBs with dimensions of 240x115x90 mm were manufactured using a hand-operated manual press as shown in Fig. 3(a). The materials used to manufacture the CSEB is sand, soil, cement and municipal solid waste incinerator fly ash with different mix ratio. A sieve with a mesh size of 4.75 mm is also used to screen the sand and MSWIFA. The soil and sand were initially mixed and the MSWIFA was then added. Further blending continues, following the addition of the cement and thorough mixing until a cohesive mix is obtained. For moulding blocks, the amount of water determined individually by the proctor test for each mix is adopted. To obtain a consistent moisture distribution, the appropriate amount of water was progressively poured into the dry mix and repeatedly rotated around [2]. The wet mixture was then poured in the press mould and any excess material removed from the mould. The combination was then manually compacted, and the block was ejected right away Fig. 3(b). After 24 hours of casting, under the wet gunny bags, the blocks were cured for 28 days and dried in the laboratory for 1 week before testing.

**Figure.3.** (a) Block making machine, (b) CSEB produced**Block Testing**

Three samples for compressive strength, three samples for water absorption, and three samples for flexural strength were examined for each variation. A compression test with a 100-ton capacity was performed and the ability to deliver uniform force to failure[17]. The CSEB were placed between two steel plate and load was given to determine the compressive strength. CSEB was immersed in water for 24hours to determine the water absorption according to IS 3495 part II. On phase I, alternating wetting–drying and sulfate attack

experiments were performed to assess durability and accordance with IS 1725, an alternate wetting–drying test was carried out. Blocks were dried in an oven at 50°C-60°C until they reached a consistent weight and the dry weight was recorded. The blocks were then immersed in room temperature water for 5 hours before being oven dried at 60°C 42 hours. Taking the blocks from the oven, scrape the wire on each side of the blocks with a force of 1.5 kgf twice. Twelve similar cycles have been performed and the blocks have been dried at 60 C till they achieve their immediate weight[7]. The dried block weight is noticed. Blocks have been oven-dried for 2days at 100 C and dry weighs have been noted. To test sulphate resistance, blocks were exposed to a sodium sulfate (Na_2SO_4) solution and then blocks have then been put within the container and the sulfate solution is progressively poured until it reaches a height of 2cm to 3cm from the base of the block's, as shown in Fig. 4. Samples were removed and weighted after 1 week from the solution [16].



Figure.4. Sulfate attack test

RESULTS AND DISCUSSION

OMC and MDD characteristics

Effect of municipal solid waste incinerator fly ash as a soil-sand mixture replacement.

Mixtures of the OMC and MDD with 0%, 7%, 14%, 21% and 28% of MSWIFA. The inclusion of MSWIFA decreases the rise MDD and increases the OMC correspondingly. With the proportion of MSWIFA from 0 to 28%, the OMC raise from 11.09% to 12.98% and the MDD reduce from 1940 kg/m³ to 1812 kg/m³.

Table.6. Phase I and Phase II test result for OMC and MDD

Phase I			Phase II		
Sample	OMC %	MDD (kg/m ³)	Mix	OMC %	MDD (kg/m ³)
IF	11.09	1940	E1	13.21	1891
IF7	11.78	1911	E2	13.47	1828
IF14	12.23	1876	E3	14.32	1767
IF21	12.51	1841	E4	14.98	1719
IF28	12.98	1812	E5	15.12	1678
			E6	15.45	1626

Effect of soil-sand mixtures of the MSWIFA particle size and quantity

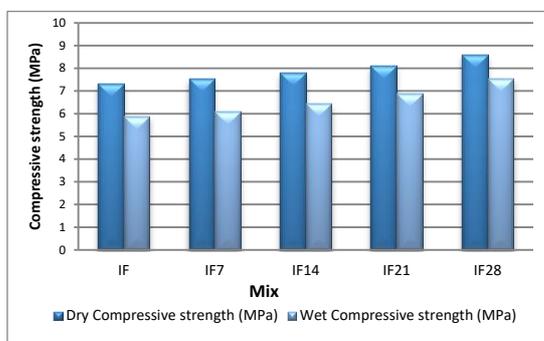
The impact on the OMC and MDD soil-sand-cement mixes and substitution of MSWIFA are shown in Table 6. The MDD decreases and OMC increases with MSWIFA substitution in comparison to the control mix, for phase II. The MDD and OMC values for 0%, 20%, 40%, 60%, 80%, and 100% replacement ratios of MSWIFA particle size between 0/4.75 mm (E) are 1891 kg/m³–1625 kg/m³ and 13.21–15.45%, respectively, when sand is substituted by MSWIFA particles with sizes ranging from 0 to 4.75 mm (E). Among 20%, 40%, 60%, 80%, and 100% replacement ratios E2 seemed to have the highest MDD value (20% of the sand was replaced by MSWIFA particles with sizes ranging from 0 to 4.75 mm).

Dry and Wet compressive strength

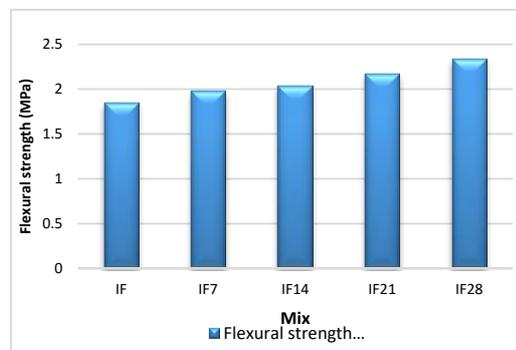
Effect of municipal solid waste incinerator fly ash as a soil-sand mixture replacement

After 28 days of curing, the dry and wet compressive strength of CSEBs is shown in Fig. 5. (a). It can be shown that adding MSWIFA to the mix increases compressive strength considerably when compared to the control sample; this finding is consistent with previous research. Table 7 with the average dry compression strength obtained in the range 7.32–8.61 MPa and the wet compressive strength achieved in the range 5.89–7.56 MPa which was corresponding to 0–28% of MSWIFA. In order to better understand the distributed size of mixes in particles, there is an obvious correlation between the coefficient of uniformity and curvatures and the compressive strength. The use of MSWIFA fines to fill the spaces between sand particles, as well as a reduction in clay content, were expected to increase cement with

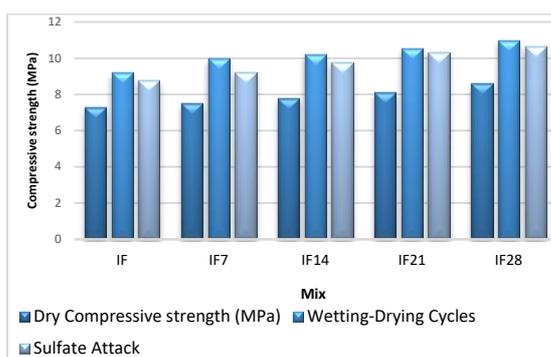
sand grain efficiency [14]. These findings are consistent with previous research that found a loss of 5–20% for soil-sand-lime-rice hush-ash mixtures[16] and a loss of 19–33% for cement-stable soil-grained blast-furnace slag blends[5], respectively, where a loss of strength ranged from 10 to 23% for samples and 30% ceramic waste. [18][19].



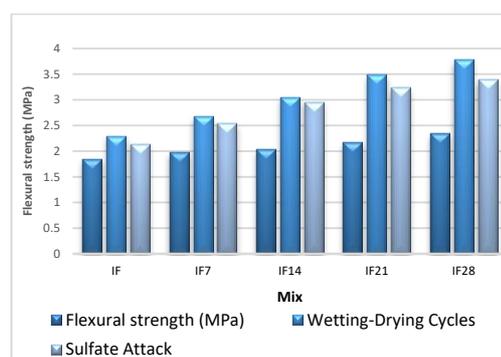
(a) Phase I block Compressive strength



(b) Phase I block Flexural strength



(c) Phase I block Compressive strength before and after sulphate attack and wetting-drying cycle



(d) Phase I block Flexural strength before and after sulphate attack and wetting-drying cycle

Figure.5. Phase I results

Effect of soil-sand mixtures of the MSWIFA particle size and quantity

The compression strength of blocks with of MSWIFA is shown in 8. The results show that blocks with size of MSWIFA lower their strength, with the proportion of substitution increasing relative to the control sample. Initially the strength of the E (manufactured from 0/4.75 mm MSWIFA) blocks increased for 20% substitution, but subsequently slowly decreased for further substitution. Due to the combined impact of pozzolanic effects and the filling effect of MSWIFA fines, the strength of E2 was increased [17] . The decrease in strength at larger replacements, might be attributed to a rise in fines content, which increases porosity, in addition to the weaker characteristics of MSWIFA compared to sand. The compressive test of block shown in Fig.6.[20].

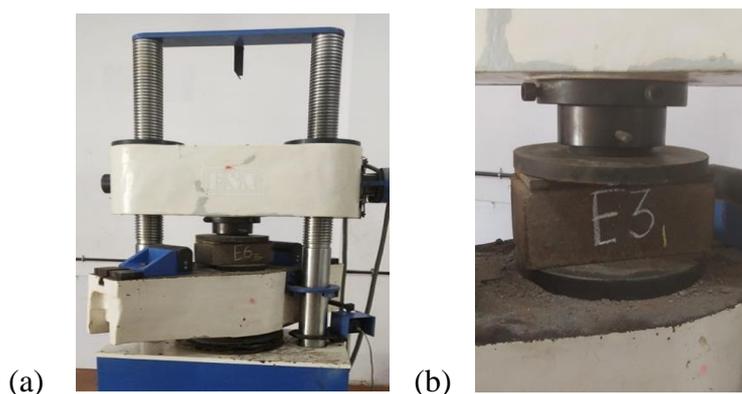


Figure.6. (a) & (b). Compressive test of block

The strength improved in E2 was predicted due to the presence of fines. The incinerator solid waste, generally is less than 75 mm reactive due to high-temperature burning[21]. These results show that the size of the MSWIFA particle has significant impact on the strength of the block. The mixture's particle size distribution was also considered in order to explain the trend in strength with the substitution rate for MSWIFA[22]. The compressive strength of E2 is generally stronger in dry and wet conditions, but in wet conditions decreases dramatically[23].

Table.7. Results of Phase I

Sample	Dry Compressive strength (MPa)	Wet Compressive strength (MPa)	Flexural strength (MPa)	Water absorption (%)	Wet-Dry strength ratio
IF	7.32	5.89	1.85	7.49	0.80
IF7	7.54	6.12	1.98	7.92	0.81
IF14	7.80	6.45	2.04	8.34	0.82
IF21	8.11	6.89	2.18	8.89	0.84
IF28	8.61	7.56	2.34	9.02	8.87

Flexural strength

Effect of municipal solid waste incinerator fly ash as a soil-sand mixture replacement

Flexural strength of blocks containing municipal solid waste incinerator fly ash was greater than the control, similar to compressive strength behaviour. The flexural strength blocks are

shown in Table 7. The greatest strength of 2.34 MPa was achieved for blocks made with 28% MSWIFA, as illustrated in Fig. 5(b). The division into two halves under a three-point load failed all blocks.

Effect of soil-sand mixtures of the MSWIFA particle size and quantity

The flexural strength of blocks containing MSWIFA is shown in Table 8 and Fig. 8. Except for the E2-20% block, the flexural strength of blocks decreases as the replacement percentage rises. The greatest flexural strength found for 20CF blocks was 2.57 MPa, which is 16% more than the control sample value. In addition to E2-20% blocks are decreasing in strength from 40% to 100% for MSWIFA. The flexural strength was clearly harmed by the use of extremely fine MSWIFA in higher proportions. In all, the CSEBs were over the required flexural strength of 0.25 MPa for load-bearing masonry. It was concluded that the elimination of fines from MSWIFA did not have any significant impact on the flexural strength. In turn, MSWIFA and its replacement percentage have impacted the strength of the block considerably. For the combination, 20% of MSWIFA in particle size 4.75 to 0 mm (20% E2) was achieved the optimal strength.

Table.8. Results of Phase II

Sample	Dry Compressive strength (MPa)	Wet Compressive strength (MPa)	Flexural strength (MPa)	Water absorption (%)	Wet-Dry strength ratio
E1	8.51	7.34	2.91	8.50	0.86
E2	7.92	7.11	2.57	9.87	0.89
E3	7.51	6.76	2.09	10.91	0.90
E4	7.25	6.54	1.98	12.21	0.90
E5	6.5	5.12	1.76	13.01	0.78
E6	5.9	4.57	1.21	14.78	0.77

Water absorption

Effect of municipal solid waste incinerator fly ash as a soil-sand mixture replacement

With the increase in the contents of MSWIFA, water absorption of the phase I blocks increases from 7.49% to 9.02%, with the dosage of municipal solid waste incinerator fly ash increasing from 0 to 28%. as shown in Fig. 7 and Table 7.

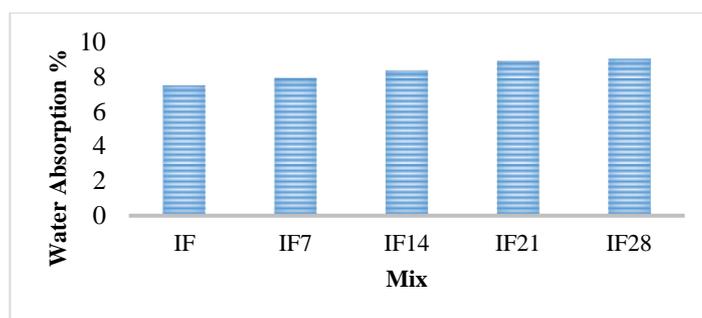


Figure.7. Water absorption results of Phase I

Effect of soil-sand mixtures of the MSWIFA particle size and quantity

Figure 8 show the effect of MSWIFA size and replacement ratio on block water absorption. The data shows that water absorption rises with increasing percentage replacement. For 0/4.75 mm (E) blocks, water absorption values of E1, E2, E3, E4, E5, E6 are higher than the control blocks. Sand may be substituted with MSWIFA in this experiment instead of soil since these changes are possible. It shows the integration of fine, MSWIFA that results in greater absorption. The wet compressive strength behaviour is confirmed by these data. As the replacement percentage ranges from 20% to 100%, the increase in water absorption values is roughly $\pm 6\%$. However, the inclusion of fines in E mixes enhanced the microstructure by pores filling and reacting with calcium hydroxide, which led to the thick matrix, reducing water absorption. This may be due to the filling of gaps with municipal solid waste that hinders block water entry, but this impact was no longer seen when replacing more than 40%. This might be due to vacuums being filled with MSWIFA fines preventing the water entry into the block, whereas this impact was no longer seen at above 40% replacement. This was mainly due to greater water absorption of finer particles than MSWIFA rougher particles. Overall, in one hand, there were little variations in water absorption when the powder content was removed. The finer the particle size of the MSWIFA, the greater the water absorption, and its dependence on particle size were verified.

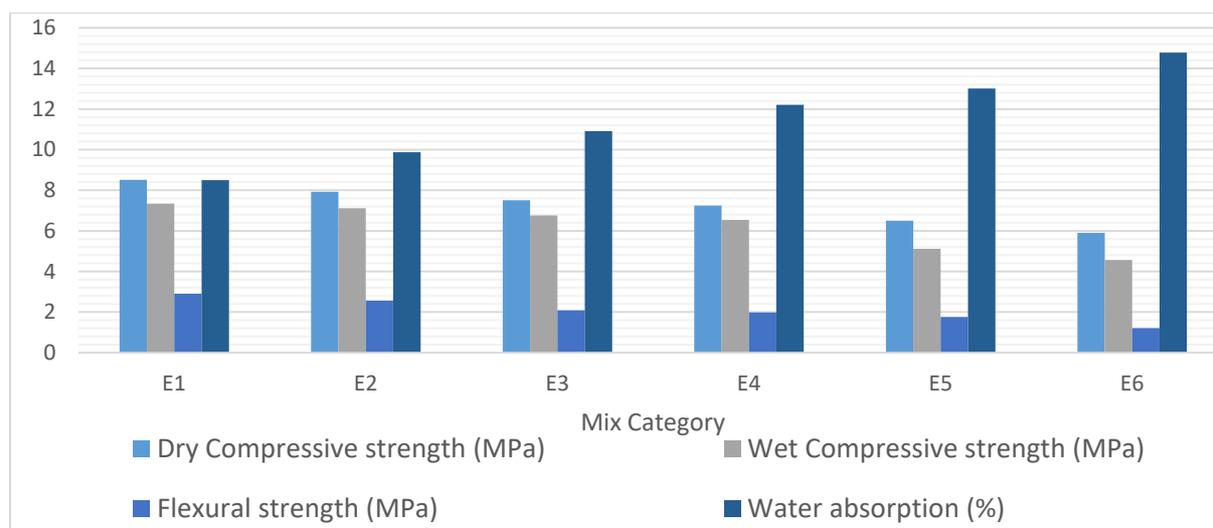


Figure.8. Phase II blocks result

Durability tests

Wetting-Drying resistance

Blocks were checked for any cracks, once testing has been completed. The little damage of surface and angle particles occurred in the entire block. However, due of strong adhesion between particles and matrices, blocks containing MSWIFA demonstrate a greater abrasion resistance. These blocks were tested for compressive and flexural strength in order to assess the mechanical performance. The results are shown in Fig. 5 (c) and (d), Table 9. Blocks made with 28% of MSWIFA attained the maximum strength of 11.01 MPa (E28). After wetting-drying tests, control block strength was increased, similarly for blocks containing MSWIFA of 7 to 28% the improvement in strength (Table 9). The compressive strength of 7-28% MSWIFA in blocks is higher than control. The strength values for blocks containing 0-28% MSWIFA were typically between 2.30 and 3.49 MPa. Also, as compared to controlling the flexural strength, the MSWIFA is increased by around from 7 to 28%.

Table.9. Sulfate Attack and Wetting-Drying cycle of Phase I blocks

Sample	Wetting-Drying Cycles		Sulfate Attack	
	Dry compressive strength (MPa)	Flexural strength (MPa)	Dry compressive strength (MPa)	Flexural strength (MPa)
IF	9.22	2.30	8.78	2.14
IF7	9.98	2.68	9.21	2.54
IF14	10.23	3.06	9.76	2.96
IF21	10.53	3.49	10.34	3.24
IF28	11.01	3.78	10.65	3.40

Sulfate resistance

On the blocks there were no obvious damages/spalling. But on control block edges and sides a small layer of floescence formed. While there was no indication of effloescence for blocks made from municipal solid waste incinerator fly ash. The results of 0–28% percent MSWIFA blocks before and after exposure to sodium sulphate (Na_2SO_4) solution are given in Fig.5 (c) and (d). The compressive strength was greater than the initial value recorded at 28 days of air curing, all blocks subjected to sulphate attack. The block with a 28% MSWIFA of 10.65 MPa has the maximum strength. After 2 cycles in Na_2SO_4 solution, the compressive strength of the blocks rises. Similarly, the strength increase following Na_2SO_4 exposure for blocks of 7–28% MSWIFA (Table 9). The flexural strength of blocks subjected to 3% Na_2SO_4 increased. As the amount of MSWIFA ranges 0–28%, strength levels range from 2.14 MPa to 3.40 MPa as shown in Fig. 5(d). With 7%, 14%, 21% and 28% of MSWIFA, the flexural strength of the blocks than the control. Usually, after sulfate has been added into cement, this reacts to gypsum & ethringite mostly extensive in nature with the portlandite and aluminium phases.

CONCLUSIONS

An in-depth experimental research was carried out to analyse the effects of municipal solid waste incinerator fly ash on the characteristics of compressed stabilized earth blocks to replace the soil-sand mix and its particle size and substitution ratios. As the percentage of MSWIFA increases, the OMC increases and MDD reduces. MSWIFA added up to 28% increased compressive (wet-dry) and flexural strengths. The strengths of sulfate exposure enhanced significantly during wetting–drying cycles due to the production of more compounds. The 28% block of municipal solid waste incinerator fly ash had the best mechanical strength and improved durability. Water absorption of blocks rises when the MSWIFA percentage increases in phases I and II, regardless of particle size. At a replacement amount of sand of 100% by fine MSWIFA, the maximum water absorption was observed (100E6). Further research will thus be necessary in order to evaluate block long-term durability. The current study has proven a possibility for the manufacturing of compressed earth blocks of soil-sand mixing and sand replaced with MSWIFA without affecting mechanical and durability performance.

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