

# Impact of high temperature on mortar mixes containing additives

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## ABSTRACT

The structures may be exposed to fire or high temperature conditionally or accidentally. Alteration in the behavior of concrete structure is prospective under the exposure of elevated temperature. There is an urge to find the materials that can resist the alteration in physiochemical and strength properties of cementitious materials under high temperature. In the present study, the effect of elevated temperature on cement mortar consisting of additives, i.e., accelerating admixtures, and stone waste, i.e., stone slurry powder, was investigated and compared with specimens at room temperature. The aim of the study was to examine the practicability of these additives under exposure to high temperatures. The mortar specimens were exposed to various temperatures, i.e., 150<sup>0</sup>C, 300<sup>0</sup>C, 450<sup>0</sup>C, and 600<sup>0</sup>C, for the duration of one hour and compared with unheated samples. The change in mass, strength, and micro-structure of mortar specimens at elevated temperature was studied. The environmental assessment and performance evaluation of various mortar mixes were also evaluated. The mass of mortar specimens reduced as the exposure temperature of specimens was raised. The residual strength of mortar increased up to a certain temperature; afterwards, it decreased. Stone slurry powder and calcium nitrate can be used individually and in combination to resist thermal changes.

**Keywords:** Accelerators; Environmental assessment; Microstructural analysis; Stone slurry powder; Strength and mass loss.

## INTRODUCTION

Fire safety of structure is a crucial aspect due to numerous terrorist attacks, war operations, and natural disasters caused by industrial production and traffic accidents. Fire jeopardizes human health, wildlife, personal, public property, and environmental damage (Bodnarova et al., 2017). The cement composites are prone to high temperature either during fire or vicinity of furnace and reactors. The behavior of cement-based materials is related to the properties and compositions of used ingredients under exposure to higher temperature. The strength and durability of cement composites significantly decrease under exposure to high temperature due to the formation of cracks and spalling, ultimately leading to deterioration and sometimes even failure of the structure. The physical structure and chemical compositions of cement composites alter under the exposure to elevated temperature (Arioz, 2007, Bing'ol and G'ul, 2009, Karahan, 2011, Shaikh and Taweel, 2015, Hager et al., 2016 & Belouadah et al., 2018). The color, strength, density, elastic modulus, and appearance of concrete structures changed under the exposure to high temperature, which results in reduction in the quality of concrete (Demirel and Kelestemur, 2010, Netinger et al., 2011, Aslani and Samali, 2014 & Baloch et al., 2018). The degradation in mechanical properties can be attributed to

physical, thermal, and chemical changes in the hydration product of cement paste and aggregates (Guerrieri et al., 2009). The advantage of concrete using as a construction material is to have significant resistance against elevated temperature. The distress in concrete occurs in the form of cracking and spalling on its surface when subjected to elevated temperatures. The introduction of industrial by-products influenced the structural properties of concrete (Turkmen and Findik, 2013 & Khan and Abbas, 2016). The physical and chemical changes due to elevated temperature affect the strength and durability of concrete. The consequence of high temperature on mechanical and durability properties had been investigated by researchers to develop the fire-resistant materials. The properties of cement paste, aggregates, cement paste aggregate bonding, presence of supplementary cementitious materials, heating rate, cooling type, loading conditions, and moisture regime were found dominating factors that influenced the fire resistance of concrete (Koksal et al., 2015). High temperature affects the structure of a building; therefore, it needs to select suitable materials and specific compositions that are important (Melichar et al., 2017).

During dimensioning of stones, a large amount of waste in various forms is generated at sites. The waste is directly disposed off to the surrounding area or in streams. The disposal of stone waste on land reduces the fertility of soil by clogging the water percolation due to the presence of its finer particles. The lifting of fine particles of stone slurry by air on drying is injurious to health and also causes environmental issues. The direct disposal of stone waste into water courses pollutes the water. The sustainable solution to disposal problem of stone waste is to utilize it in the construction industry as a substitution to cementitious materials. Therefore, utilization of stone powder and other industrial by-products in cement based materials helps produce environment friendly end products with conservation of natural resources, reduction of landfill issues, and development of sustainable materials (Rana et al., 2015, Tharrini and Ramasamy, 2016, Singh et al., 2017, Khaliq and Taimur, 2018, Sánchez et al., 2018 & Devi et al., 2018b; 2019).

Concrete is the important building materials in this modern building techniques (Güçlüer, 2021). Admixtures are the chemicals used either before or during the blending of ingredients to modify the specific characteristics of cementitious materials. The chemical admixtures are air entrainer, accelerators, water reducer, and retarder types and have their applications. Accelerating admixtures accelerate the stiffening and early age strength of mortar and concrete. The most commonly used accelerating admixture is calcium chloride, which is not used in reinforced structures because of its corrosive behavior. Non-chloride accelerating admixtures have also been used in concrete to avoid corrosive situations. Calcium nitrate (CN) and triethanolamine (TEA) have been used in the present work (Aggoun et al., 2008, Devi et al., 2018a & Devi et al., 2020).

The weight and relative strength of specimens reduced with temperatures. The influence of high temperature on the strength of concrete with river gravel aggregates was more pronounced (Arioz, 2007). The addition of steel fiber in mortar limits the damage and cracking when subjected to high temperature; polypropylene fiber created porosity and limited the pore pressure, consequently reducing the cracking (Ezziane et al., 2011). The pore area fraction increased, and hydrated paste area fraction decreased with a rise in temperature, which resulted in the degradation of microstructure and the affected strength of the mortar. The critical temperature for the change in properties of mortar specimens containing fly ash and metakaolin was 400°C (Nadeem et al., 2013). The loss in strength properties of normal strength high-performance concrete due to elevated temperature was similar to high strength high-performance concrete. Concrete with fiber increased the residual strength. The explosive spalling was prevented by polypropylene fiber (Peng et al., 2014). The lightweight concrete with vermiculite had good performance at a higher temperature (Koksal et al., 2015). The inclusion of limestone powder (LP) reduced the creep of the concrete at a higher temperature, and this reduction increased at the higher replacement of cement by limestone powder. The variation of internal relative humidity was delayed by LP for all the mix proportions (Wang et al., 2015). The silica fume (SF) based concrete had poor performance, while slag-based concrete showed good performance against the high temperature (Khan and Abbas, 2016). Crushed rock dust as filler materials improved the internal microstructure. The microstructure was still dense, and fewer micropores were present at 200°C, and little deterioration of

microstructure bonding was observed at 400<sup>0</sup>C temperature. The significant damage takes place at 600<sup>0</sup>C temperature due to enlarged cracks (Kumar and Ram, 2019). The cement matrix consisting of calcined diatomite powder was compact, and bonding between cement paste and sand was intact at ambient temperature. At 400<sup>0</sup>C temperature, cement matrix was still compact and strong but weakened the interface zone between cement paste and sand. After 800<sup>0</sup>C, critical changes in the morphology of CSH were observed, and they weakened the bonding at 1000<sup>0</sup>C (Saridemir et al., 2020). The alkali-activated mortar with waste ceramic powder (WCP) at 70% showed more stable surface at high temperatures than at 50% WCP (Shah and Huseien, 2020). The ettringite and monosulphate phase decomposed, and it was absent at 300<sup>0</sup>C temperature. At 550<sup>0</sup>C and 900<sup>0</sup>C, dense structure of hydration products becomes gradually destroyed and loose (Lin et al., 2020). A simplified relationship between external restraint degree and allowed temperature difference was developed (Xin et al., 2018)

## Research Significance

In the present work, admixtures, i.e., calcium nitrate (CN) and triethanolamine (TEA) as additive of accelerating nature, and stone slurry powder (SSP), waste from stone industries (replacement of cement), have been used in cement mortar to study its behavior at elevated temperature. Residual strength of specimens was defined as strength measured after cooling the specimens. The influence of high temperatures (150<sup>0</sup>C, 300<sup>0</sup>C, 450<sup>0</sup>C, and 600<sup>0</sup>C) with an exposure duration of 60 minutes on cement mortar consisting of additives in terms of variation in mass, strength, and microstructural characterization was investigated. The ecological and economic analysis was also carried out. The purpose was to check the feasibility of these materials individually and in combination with mortar at high temperatures. The aim of combining admixtures and SSP in cement mortar was to make cost-effective and eco-friendly construction materials that can also resist the changes under elevated temperature effectively.

## EXPERIMENTAL PROGRAM

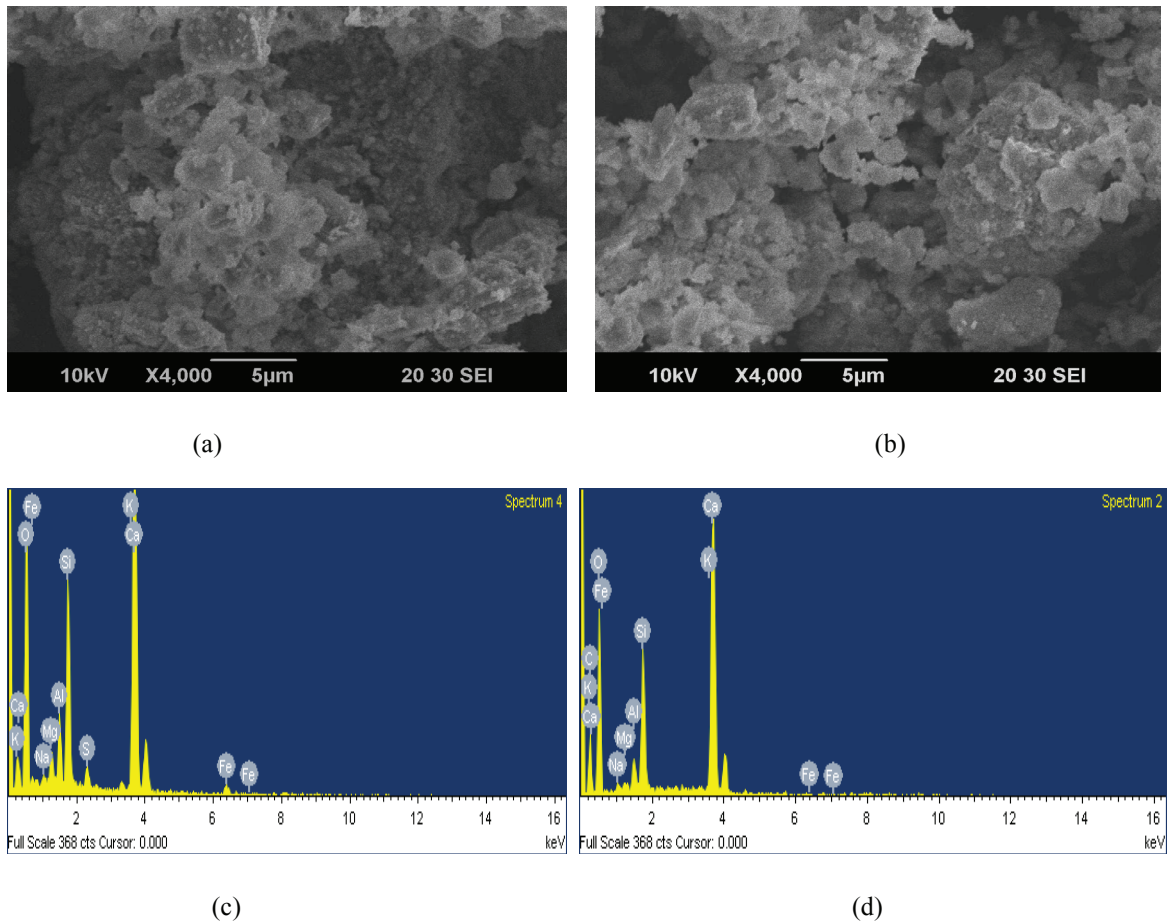
### Materials and Mix Proportions

To study the behavior of cement mortar consisting of triethanolamine, calcium nitrate, and stone slurry powder under exposure to elevated temperature, twenty-two mortar mixes of different mix proportions were prepared. Ordinary Portland cement (OPC) of 43 grade (fineness of 4%, 27.5% consistency, specific gravity of 3.12, and 28 days strength of 44.5 MPa) conforming to Indian Standard (IS): 8112-1989 and coarse sand (fineness modulus of 3.17, zone II, specific gravity 2.62) conforming to IS: 383-2016 were used to study the consequence of elevated temperature on mass and residual strength of cement mortar (Devi et al., 2018). The cement mortar was prepared in accordance with IS: 4031-1989 (Part-6). Tap water was used during the whole experimentation. Stone slurry powder (white in color and specific gravity 2.72), waste generated during dimensioning of stones, was used as a partial substitution to cement. SSP was procured from Kota stone (calcareous sedimentary rock), Kota, Rajasthan, India, in slurry form, dried under sunlight and crushed to fine particles, and utilized as replacement of cement. Accelerators, i.e., calcium nitrate tetra-hydrate purified [Ca(NO<sub>3</sub>)<sub>2</sub>.4H<sub>2</sub>O] and triethanolamine LR having the chemical formula [C<sub>6</sub>H<sub>15</sub>NO<sub>3</sub>], were used as cement additives in the mortar and procured from the local supplier (Devi et al., 2019b and Devi et al., 2020).

**Table 1.** Chemical composition of OPC and SSP (Devi et al., 2020).

Chemical composition (%)	CaO	SiO <sub>2</sub>	Al <sub>2</sub> O <sub>3</sub>	MgO	Na <sub>2</sub> O	FeO	K <sub>2</sub> O
OPC	60.29	21.42	5.91	2.65	0.64	4.81	1.11
SSP	49.78	17.01	2.92	0.61	0.88	0.14	0.42

The chemical composition of OPC and SSP from energy dispersive X-ray spectroscopy (EDS) has been tabulated in Table 1, and their scanning electron microscopy (SEM) and EDS images have been illustrated in Figures 1 (a), (b), (c), and (d), respectively. The particle size of SSP was finer as compared to cement as observed from SEM micrographs. The irregular shape of SSP particles was also observed from SEM image. EDS images showed the presence of elements, i.e., calcium, silica, aluminum, iron, sodium, etc. From EDS analysis, it was observed from the chemical composition of cement and SSP that they were quite similar. With calcium being a major element, SSP was calcite in nature, contributing to binding capabilities by reacting with tricalcium aluminate ( $C_3A$ ) to form calcium carbon aluminate.



**Figure 1.** (a) SEM of PC; (b) SEM of SSP; (c) EDS of PC; (d) EDS of SSP.

## Mixture Proportions

CN (0%, 1% and 2%), TEA (0%, 0.025%, 0.050%, and 0.1%), and SSP (0%, 2.5%, 5%, 7.5%, and 10%) have been used in mortar mixes, and twenty-two various mixes of mortar were studied as tabulated in Table 2. The quantity of cement, sand, and water was  $575 \text{ kg/m}^3$ ,  $1725 \text{ kg/m}^3$ , and  $158.25 \text{ kg/m}^3$ , and quantities of CN, TEA, and SSP were varied according to the dosages. The water-binder ratio was kept constant at 0.275, and cement-sand ratio was 1:3 throughout the experimentation.

**Table 2.** Mix proportions of various mixes of cement mortar (w/b .275).

Mix No.	Cement, kg/m <sup>3</sup>	Sand, kg/m <sup>3</sup>	water, kg/m <sup>3</sup>	SSP, kg/m <sup>3</sup>	CN, kg/m <sup>3</sup>	TEA, kg/m <sup>3</sup>
F0	575	1725	158.1	0	0	0
F1	560.62	1725	158.1	14.73	0	0
F2	546.25	1725	158.1	28.75	0	0
F3	531.88	1725	158.1	43.13	0	0
F4	517.5	1725	158.1	57.5	0	0
F5	575	1725	158.1	0	0	0.14
F6	575	1725	158.1	0	0	0.29
F7	575	1725	158.1	0	0	0.58
F8	575	1725	158.1	0	5.75	0
F9	575	1725	158.1	0	11.5	0
F10	575	1725	158.1	0	5.75	0.14
F11	575	1725	158.1	0	11.5	0.14
F12	546.25	1725	158.1	28.75	0	0.29
F13	531.88	1725	158.1	43.13	0	0.58
F14	546.25	1725	158.1	28.75	5.75	0
F15	531.88	1725	158.1	43.13	5.75	0
F16	546.25	1725	158.1	28.75	11.5	0
F17	531.88	1725	158.1	43.13	11.5	0
F18	546.25	1725	158.1	28.75	5.75	0.58
F19	531.88	1725	158.1	43.13	5.75	0.58
F20	546.25	1725	158.1	28.75	11.5	0.29
F21	531.88	1725	158.1	43.13	11.5	0.29

## Methods

The cement mortar cubes were used to investigate the impact of elevated temperature on mass, residual strength, and microstructure. The cement mortar cubes of standard size of 70.6 mm x 70.6 mm x 70.6 mm were prepared as per IS: 4031-1988 (Part-6). After completing 28 days of water curing, mortar specimens were withdrawn, dried, and then held in an electric furnace for required temperatures for 60 minutes; after turning off the furnace, specimens were annealed until the room temperature attained. After cooling, mass and strength of the specimens were measured (Koksai et al., 2015; Cheyrezy et al., 2001 & Netinger et al., 2011). Initially, mortar specimens were exposed to higher temperature 900-1000<sup>0</sup>C (after literature review), but the specimens burst at high temperatures; thereby,

temperature was restricted to 600°C after trials. To examine the effect of temperatures, temperature from room temperature to 600°C, four intervals of temperatures were selected. The temperature variation was 150°C, 300°C, 450°C, and 600°C, and the rate of heating was kept 100°C/hr in the furnace. The residual strength of mortar at different temperatures was compared with the unheated mortar specimens. The mass loss of specimens was determined by the following equation:

$$Massloss = \frac{(W_1 - W_2)}{W_1} \times 100 \quad (1)$$

where  $W_1$  = initial weight of mortar specimen;  $W_2$  = weight of specimen after exposure to elevated temperature.

### Ecological and Economical Analysis of Mortar

In the construction industry, cement is the main ingredient of concrete, which is responsible for the emission of CO<sub>2</sub> after coarse aggregates and fine aggregates. Coarse aggregates have a higher contribution in releasing CO<sub>2</sub> during all processing than fine aggregates, because the former are crushed not graded. Flower and Sanjayan (2007) suggested that emission due to chemical admixtures is neglected due to very small quantity as compared to other ingredients; but in this study, their effects are considered except TEA, because its dosage is very low (<2l/m<sup>3</sup>). Also, batching, transportation, and placing of concrete have less contribution to CO<sub>2</sub> emission (Long et al., 2017). Since the use of cement leads to harass the environment, therefore, it needs a replacement partially or fully with suitable industrial by-products to conserve the natural resources and emission of greenhouse gases. This small step leads to the development of sustainable construction. The ecological and economic aspects are two factors that can be used to assess sustainability. The ecological aspect of mortar and concrete can be analyzed in terms of embodied energy (EE) and embodied carbon dioxide (ECO<sub>2</sub>), and economic analysis can be analyzed in terms of cost. EE and ECO<sub>2</sub> are defined as the consumption of energy and emission of CO<sub>2</sub> during the assembly, transportation, installation, and disassembly of a material/product. The values for EE and ECO<sub>2</sub> for cement, sand, coarse aggregates, water, CN, TEA, and SSP have been taken from the literature (Gupta et al., 2019 & Siddque et al., 2019) and have been given in table 3.

**Table 3.** EE, ECO<sub>2</sub>, and cost of raw materials.

Materials	Cement	Sand	Coarse aggregates	Water	CN	TEA	SSP
EE, MJ/kg	4.8	0.081	0.0083	0.2	0.1368	-	-
ECO <sub>2</sub> , kgCO <sub>2</sub> /kg	0.93	0.0051	0.0008	0.0008	0.481	-	-
Cost, USD	0.084	0.014	0.025	0.0007	5.92	14.59	-

Note that emission and cost related to the transportation of raw materials and production of mortar have not been considered. The price of materials was taken as market values of Kurukshetra, India, only. These values are used to calculate the value of EE, ECO<sub>2</sub>, and cost of mortar by using the following equation:

$$EE / ECO_2 / Cost = \sum g_i m_i \quad (2)$$

where  $g_i$  = EE per unit mass of materials and  $m_i$  indicate the mass of mortar ingredients  $i$  per unit cubic meter.

## Performance Evaluation

The quantitative analysis can be used to describe the performance of additives in mortar rather than qualitative terms. If the performance of plain mortar mix is considered as 1.0, then the performance of mortar with additives will be less than or greater than 1, which signifies the inferior or superior performance with reference to plain mix, respectively, in different aspects. The performance of additives, i.e., SSP, CN, and TEA, individually, will be evaluated with respect to plain mortar mix (Kayali and Ahmed, 2013)

## 2.6 Micro-Structure Analysis

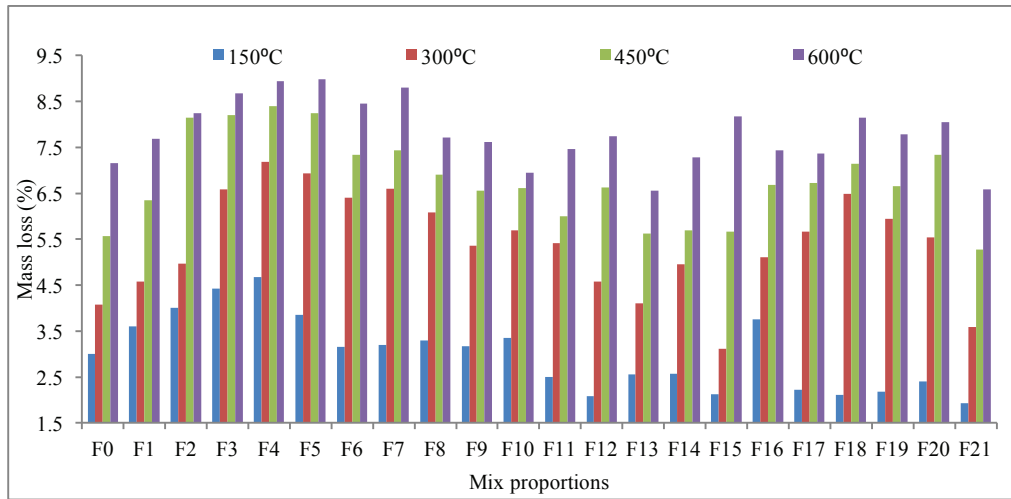
The selected mix proportions were selected to study the microstructural properties of cement mortar containing CN, TEA, and SSP at temperatures, i.e., room temperature and 300°C. The fractured samples of selected mortar cubes have been collected after strength test, and SEM analyses were conducted.

# RESULTS AND DISCUSSION

## Change in Mass

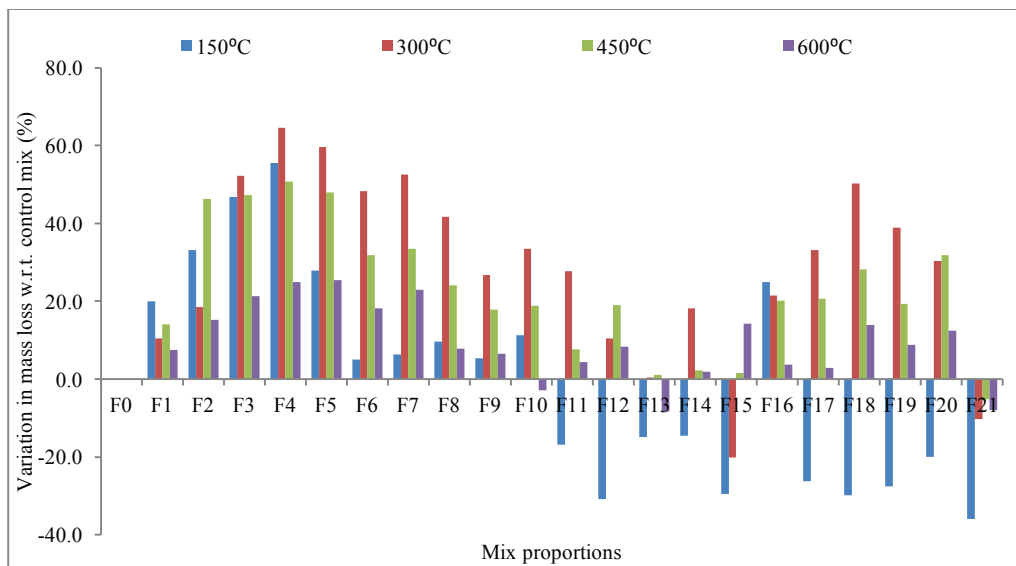
The effect of high temperature on mass loss of mortar has been studied and shown in Figures 2 (a) and (b). The mortar specimens were exposed to room temperature, 150°C, 300°C, 450°C, and 600°C, for the duration of 60 minutes to study their impact on mass of mortar. A significant loss in mass was observed as the temperature was raised. The mass loss varied from 1.93% to 4.68%, 3.59% to 7.18%, 5.28% to 8.4%, and 6.59% to 8.94%, respectively, at 150°C, 300°C, 450°C, and 600°C temperatures, respectively. Mixes F4 and F21 had the maximum and minimum mass loss at all the temperatures, as observed from Figure 2 (a). The mass loss of all the mix proportions increased with the temperature. The enhancement in mass loss of stone slurry powder may be attributed to the increase of voids in concrete (Khan and Abbas 2016). The reduction in mass may be attributed to the evaporation of water from cement paste and the formation of air voids (Demirel and Kelestemur 2010). The mass loss reduced with temperature due to release of physical or chemical bound water in mortar specimens consisting of SSP. After 450°C temperature, mass losses were due to vaporization of capillary water followed by runoff adsorbed and inter layer water. The mass loss occurred predominantly at 300–450°C; afterwards, it occurred at slow rate. The mass loss of heated specimen occurred due to eviction of hydrated water from hardened cement matrix and formed air voids in the concrete. The reduction in mass of mortar specimens confirmed the deterioration of structural integrity. The mass loss in mortar specimens below 450°C temperature may be due to evaporation of free water and capillary water. Above 450°C temperature, decomposition of CSH gel takes place, and hydrated water in CSH gel release results in mass loss of mortar samples (Kumar and Ram, 2019; Saridemir et al., 2020).

Figure 2 (b) showed the percentage variation of mass loss of cement mortar with reference to plain mix. Addition of CN, SSP, and TEA increased the mass loss as compared to plain mix at all high temperatures and also increased with the rise in temperature except for combination of additional materials at 150°C temperature. Addition of CN (1%) increased the mass loss 9.6%, 41%, 24%, and 8% at 150°C, 300°C, 450°C, and 600°C, respectively, and CN (2%) increased the mass loss by 5%, 26%, 18%, and 6% at 150°C, 300°C, 450°C, and 600°C,



**Figure 2.** (a) Mass loss of studied mixes at different temperature.

Respectively, in proportion to plain cement mortar. The inclusion of SSP enhanced the mass loss from 20% to 55%, 10% to 64%, 14% to 51%, and 7% to 25% at 150°C, 300°C, 450°C, and 600°C, respectively, with reference to plain mortar. The increment in mass loss due to TEA varied from 5% to 28%, 48% to 60%, 31% to 48%, and 18% to 25% at 150°C, 300°C, 450°C, and 600°C temperatures, respectively, with respect to mortar without any additives, i.e., F0. The reduction in mass loss for the combination of accelerators and SSP varied from 14% to 36%, except for mixes F10 and F16 at 150°C temperature. For mixes F10 and F16, mass loss increased by 11% and 25% in comparison to control mix at 150°C temperature. The mass loss reduced by 10%, 5%, and 8% at 300°C, 450°C, and 600°C, respectively, for mix proportion F21. The mix consisting of combination of accelerators and SSP increased the mass loss, and this increase in loss varied from 0.5% to 37.13% except for F15 at 300°C, 1% to 28% at 450°C, and 3% to 14% at 600°C, respectively, as shown in Figure 2 (b).

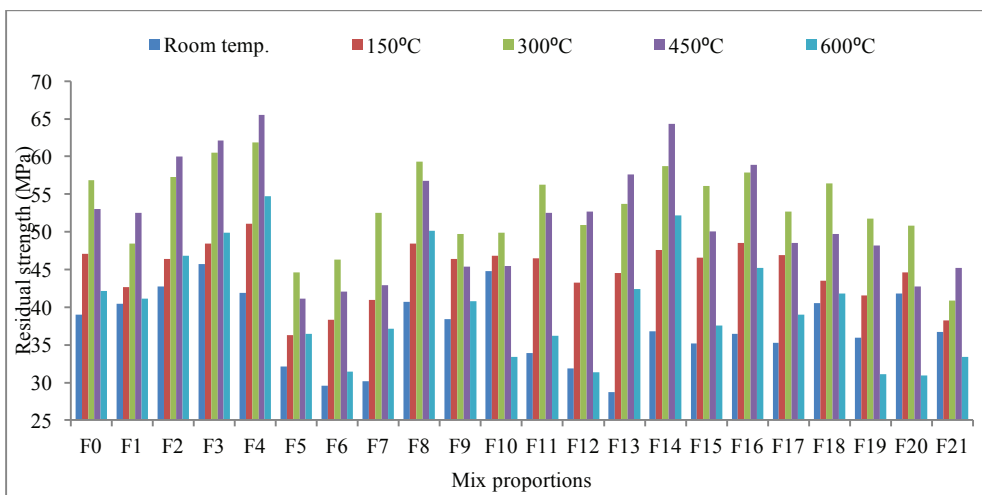


**Figure 2.** (b) Percentage variation of mass loss w.r.t. control mix at different temperatures.



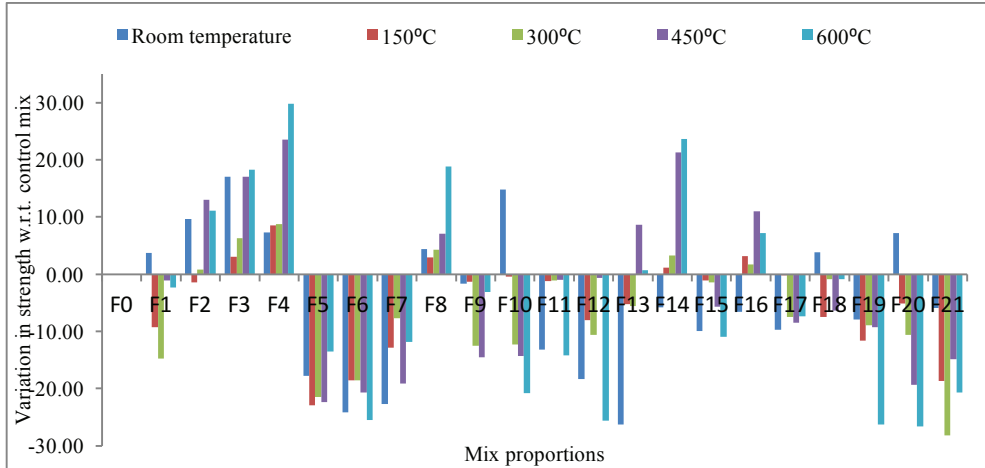
## Change in Strength

The influence of SSP, CN, and TEA individually and in combination on cement mortar subjected to different temperatures, i.e., room temperature, 150°C, 300°C, 450°C, and 600°C, has been studied and shown in Figures 3 (a), (b), and (c). For mixes F1, F2, F3, F4, F12, F13, F14, and F16, residual strength increased up to a temperature of 450°C and then decreased at 600°C, while the residual strength increased up to 300°C; afterwards, the strength reduced with temperature for the rest of mixes. An increase in residual strength up to a certain temperature may be attributed to closer configuration of cement paste after water vaporization (Aydın 2008). The increase in strength was due to expansion of moisture molecules that separate the C-S-H (calcium-silicate-hydrate) layer due to the increase in temperature and weaken Vander Waal's physical forces within the cement composites. The moisture evaporation and reduction of distance between C-S-H gel layers result in enhancement in strength (Cheyrez et al., 2001). The increase in strength may be due to acceleration of hydration process, while reduction was because of decomposition of cement paste (Khan and Abbas 2016). The reduction in strength after a certain temperature was because of degradation of C-S-H gel (Koksal et al., 2015). A sudden drop in residual strength was observed after saturation/threshold temperature, and this may be attributed to loss of water, which reduced calcium hydroxide content and formation of micro-cracks (Demirel and Kelestemur 2010). The loss in strength beyond certain temperature may be because of physical, chemical, and hydrothermal changes due to elevated temperature (Khaliq and Taimur 2018). Pachta et al. (2018) observed that the highest residual strength at 400°C temperature after strength decreased gradually, but it was high at 600°C temperature of lime-based mortar. The increase in strength may be due to the reaction that takes place as temperature increased, assisting the steam or liquid circulation due to porous structure, as reported by Pachta et al. (2018). The residual strength of calcined clay limestone cement paste increased with the increase in temperature up to 300°C due to internal claving effect. The increase in residual strength may be because high temperature and high pressure promoted the unhydrated binder particles to continue to react because of internal claving effect. The strength improvement at higher temperature was generally associated with internal claving effect, as a result of flow of stream produced due to elimination of capillary, adsorbed, and chemically bound water in matrix at high temperature. The autoclave environment enables transport of moisture, so that further hydration of unhydrated cement becomes feasible (Llin et al., 2020). The variation in results may also be due to different exposure temperature, heating duration, curing condition, and nature of specimens. The residual strength of various mixes of specimens varied from 30.17 MPa to 45.69 MPa, 36.27 MPa to 51.08 MPa, 40.86 MPa to 61.84 MPa, 41.16 MPa to 65.54 MPa, and 30.94 MPa to 54.73 MPa at room temperature, 150°C, 300°C, 450°C, and 600°C, respectively.



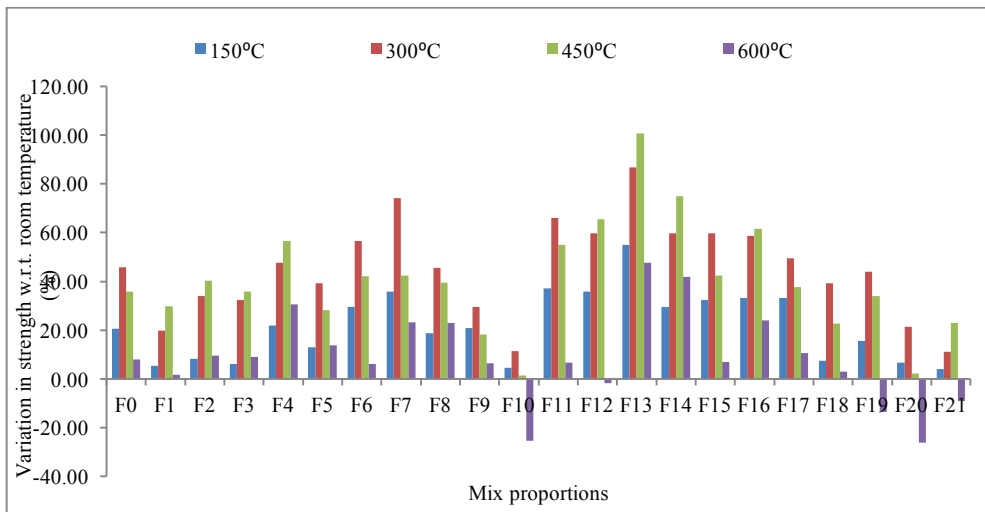
**Figure 3.** (a) Residual strength of various cement mortar mixes at different temperatures.

Figure 3(a) illustrated that the residual strength of mortar enhanced up to a certain temperature and beyond that strength reduced with elevated temperature. Figure 3 (b) showed the percentage variation in the strength of distinct mixes of mortar at high temperatures with reference to control mix. The increase in strength of mortar consisting of CN, TEA, and SSP alone and in combination for all mixes varied from 3.7% to 17%, 1% to 8.6%, 1% to 8.7%, 7% to 23.5%, and 0.6% to 30%; and reduction varied from 1.6% to 26.3%, 1% to 23%, 1.4% to 28.2%, 1% to 22%, and 1% to 26.7% at room temperature, 150<sup>0</sup>C, 300<sup>0</sup>C, 450<sup>0</sup>C, and 600<sup>0</sup>C temperatures, respectively, with respect to F0 as shown in Figure 3 (b).



**Figure 3. (b)** Percentage variation of strength of mortar mixes at different temperatures w.r.t. control mix.

Figure 3(c) showed the percentage variation of strength of mortar at different temperatures with respect to the unheated specimens, i.e., room temperature. The increase in residual strength of cement mortar consisting of additives varied from 4% to 37%, 11% to 86%, and 2% to 100% at 150<sup>0</sup>C, 300<sup>0</sup>C, and 450<sup>0</sup>C temperatures for all mixes in proportion to room temperature. At 600<sup>0</sup>C temperature, the strength was lower than control mix for mixes F10, F12, F19, F20, and F21, as shown in Figure 3(c).



**Figure 3. (c)** Percentage variation of strength of mortar mixes at various temperature w.r.t. room temperature.

## Ecological and Economical Analysis of Mortar

In the present study, the influence of CN, TEA, and SSP on the EE,  $\text{ECO}_2$ , and cost of mortar specimens with different mix proportions has been evaluated and is given in table 4. EE,  $\text{ECO}_2$ , and cost of mortar per unit strength at 28 days were also evaluated. The values of EE,  $\text{ECO}_2$ , and cost varied from 2724.37 to 2932.92  $\text{MJ/m}^3$ , 503.57 to 549.21  $\text{kgCO}_2\text{e/m}^3$ , and 67 to 141  $\text{USD/m}^3$  for all the mix proportions of mortar. It has been observed from table 4 that the use of EE,  $\text{ECO}_2$ , and cost reduced with the use of SSP. The addition of CN slightly increased the ecological impact on the environment. Since the quantity of TEA was very little, therefore, its effects on these factors are negligible. The percentage variation in EE,  $\text{ECO}_2$ , and cost with or without unit strength has been illustrated in Figure 4.

**Table 4.** Ecological and economic analysis of mortar.

Mix No.	EE ( $\text{MJ/m}^3$ )	$\text{ECO}_2$ ( $\text{kg CO}_2\text{e/m}^3$ )	Cost ( $\text{USD/m}^3$ )	EE/28 D-CS	$\text{ECO}_2/28$ D-CS	Cost/28 D-CS
F0	2931.35	543.67	71	75.09	13.93	1.8
F1	2862.32	530.3	70	71.1	13.17	1.7
F2	2793.35	516.94	69	65.28	12.08	1.6
F3	2724.37	503.57	68	59.63	11.02	1.5
F4	2655.35	490.2	67	60.69	11.2	1.5
F5	2931.35	543.67	73	91.32	16.94	2.3
F6	2931.35	543.67	75	99.03	18.37	2.5
F7	2931.35	543.67	79	96.84	17.96	2.6
F8	2932.13	546.44	105	73.78	13.75	2.6
F9	2932.92	549.21	139	78.42	14.68	3.7
F10	2932.13	546.44	107	91.97	17.14	3.4
F11	2932.92	549.21	141	101.98	19.1	4.9
F12	2793.35	516.94	73	82.42	15.25	2.2
F13	2724.37	503.57	76	60.83	11.24	1.7
F14	2794.13	519.7	103	78.14	14.53	2.9
F15	2725.16	506.34	102	77.09	14.32	2.9
F16	2794.92	522.47	137	76.64	14.33	3.8
F17	2725.94	509.1	136	77.31	14.44	3.9
F18	2794.13	519.7	111	69.77	12.98	2.8
F19	2725.16	506.34	110	75.38	14.01	3.1
F20	2794.92	522.47	141	70.9	13.25	3.6
F21	2725.94	509.1	140	71.83	13.42	3.7

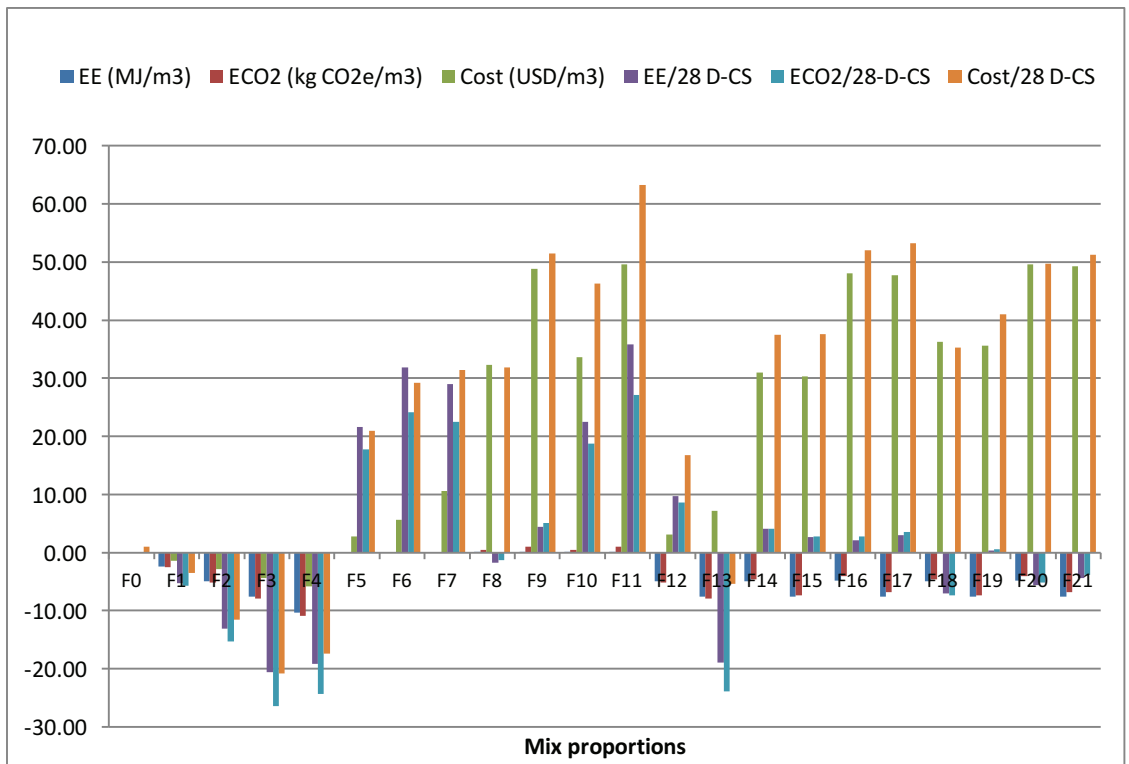


Figure 4. Percentage variation in ecological and economic parameter.

The values of EE/28D-CS, ECO<sub>2</sub>/28D-CS, and cost/28D-CS varied from 59.63 to 102, 11.02 to 19.01, and 108.72 to 357 for all the mix proportions. Figure 4 depicted that the inclusion of SSP in mortar reduced the energy consumption, emission of CO<sub>2</sub>, and cost of mortar per unit strength. Therefore, the exercise of SSP in mortar required less energy, low emission of CO<sub>2</sub>, and low cost for the unit strength, whereas the addition of chemical admixtures required high energy, more emission of CO<sub>2</sub>, and high cost for the unit strength of mortar.

### Performance index

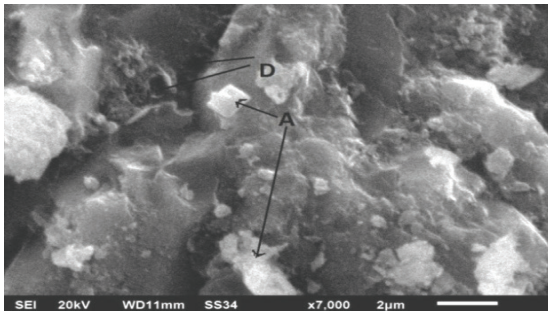
The performance index of mortar of different mix proportions was evaluated and has been given in table 5. The performance of mortar of plain mix was taken as 1.0. The values greater than 1.0 taken as superior performance and lower value indicate the poor performance with reference to control mix. The mixes F3 and F13 have the superior and interior performance of mortar specimens compared to plain mix at room temperature. Similarly, the mix F4 had the superior performance among all the mix performances at all temperatures.

**Table 5.** Performance index of different mixes of mortar.

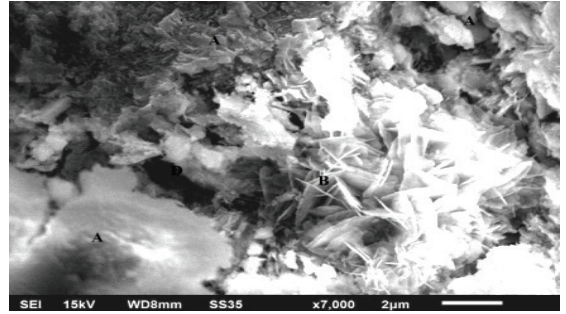
Mix No.	Room temp.	150°C	300°C	450°C	600°C
F0	1.00	1.00	1.00	1.00	1.00
F1	1.04	0.91	0.85	0.99	0.98
F2	1.10	0.99	1.01	1.13	1.11
F3	1.17	1.03	1.06	1.17	1.18
F4	1.07	1.09	1.09	1.24	1.30
F5	0.82	0.77	0.79	0.78	0.87
F6	0.76	0.81	0.81	0.79	0.74
F7	0.77	0.87	0.92	0.81	0.88
F8	1.04	1.03	1.04	1.07	1.19
F9	0.98	0.99	0.87	0.86	0.97
F10	1.15	1.00	0.88	0.86	0.79
F11	0.87	0.99	0.99	0.99	0.86
F12	0.82	0.92	0.89	0.99	0.74
F13	0.74	0.95	0.94	1.09	1.01
F14	0.94	1.01	1.03	1.21	1.24
F15	0.90	0.99	0.99	0.94	0.89
F16	0.93	1.03	1.02	1.11	1.07
F17	0.90	1.00	0.93	0.92	0.93
F18	1.04	0.93	0.99	0.94	0.99
F19	0.92	0.88	0.91	0.91	0.74
F20	1.07	0.95	0.89	0.81	0.73
F21	0.94	0.81	0.72	0.85	0.79

### Micro-Structural Properties

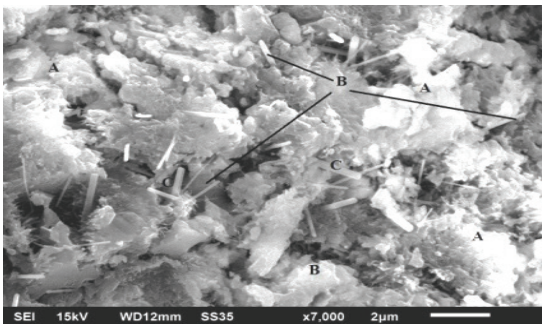
The microstructural properties of selected mix proportions were studied using SEM techniques at room temperature and 300°C temperature. SEM technique has been practiced to get the information about the morphology and crystalline structure of mortar specimens. It has been observed from Figure 5 (i-v) that C-S-H gel was spread over the matrix, which helped in strength enhancement. For mix proportions F8 and F14, dense and compact matrix was formed at all temperatures, and the presence of a lower amount of voids was observed for unheated specimens.



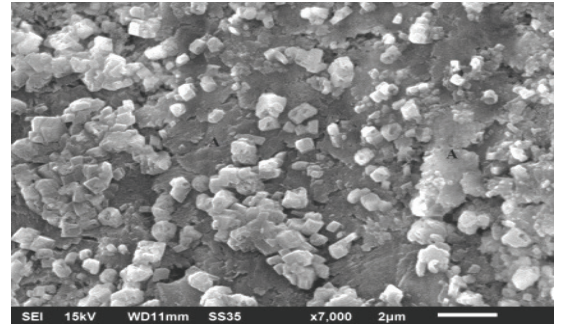
i (a)



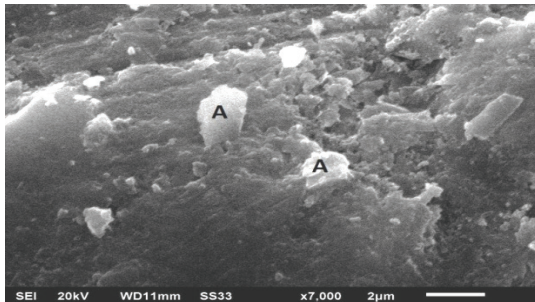
i (b)



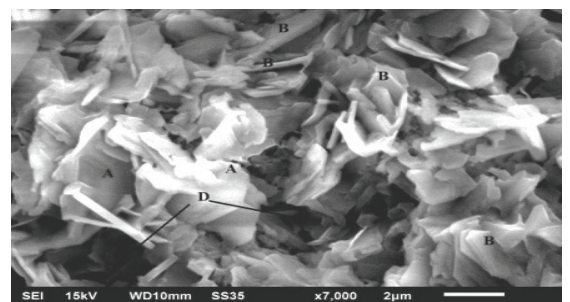
ii (a)



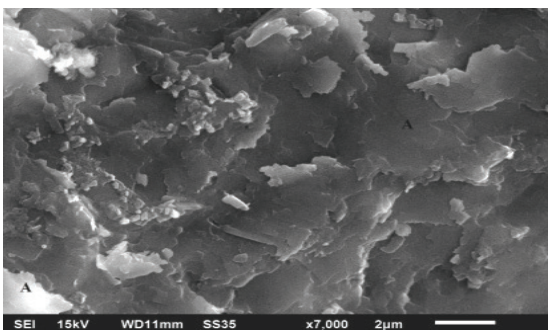
ii (b)



iii (a)



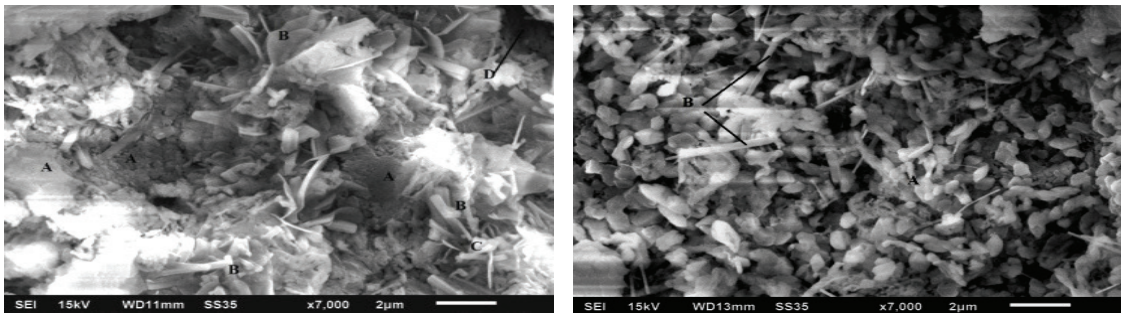
iii (b)



iv (a)



iv (b)



v (a)

v (b)

where a = specimens at room temperature, b= specimens at 300°C temperature and \*A= C-S-H, B= CH, C=ettringite and D = voids

**Figure 5.** SEM images of optimized mix proportions of (i) F0; (ii) F4; (iii) F8; (iv) F11 and (v) F16.

Compared to room temperature, specimens showed quite loose microstructure with pores and cracks. For F16 and F4 mixes, the presence of ettringite can be seen even at room temperature in comparison to a temperature at 300°C. For mix F4, dense matrix was formed, and the presence of C-S-H gel can be seen as compared to other mixes. The presence of cracks can be seen in fig. 5 i (b) at 300°C as compared to other mixes at the same temperature. At 300°C temperature, quite dense and compact microstructure was observed for mixes F4 and F11 as compared to that of mixes F8 and F16, and also visible pores were present for later mixes. Also, dense structure at room temperature is gradually destroyed and became loose. The denseness and fewer micropores may be due to steady dehydration. For mixes F4 and F16, some round shape structure is observed that dicalcium silicate may be formed by the decomposition of calcium silicate hydrate at high temperature, similar to what was reported by Lin et al. (2020).

## CONCLUSION

Stone slurry powder and admixtures were used in cement mortar to examine the behavior of high temperature on the mass, residual strength, and microstructure properties of specimens with different mix proportions. The heating temperatures were kept at 150°C, 300°C, 450°C, and 600°C for 60 minutes, and results were compared with the unheated sample. The following outcomes were found from the study:

- The mass loss of mortar consisting combination of accelerator and SSP decreased with the temperature due to the evaporation of free water. The reduction in mass loss varied from 1.93% to 4.68%, 3.59% to 7.18%, 5.28% to 8.4%, and 6.59% to 8.94%, respectively, at 150°C, 300°C, 450°C, and 600°C temperatures for all the mix proportions.
- The residual strength of mortar specimens using calcium nitrate and triethanolamine increased with the rise in temperature up to 300°C, which may be because of the acceleration of the hydration process; after that, strength decreased due to decomposition of CSH as seen in the SEM images. The residual strength of mortar with various mixes varied from 30.17 MPa to 45.69 MPa, 36.27 MPa to 51.08 MPa, 40.86 MPa to 61.84 MPa, 41.16 MPa to 65.54 MPa, and 30.94 MPa to 54.73MPa at room temperature, 150°C, 300°C, 450°C, and 600°C, respectively.
- Use of SSP increased residual strength of mortar specimens up to 450°C due to internal claving effect; beyond this, it decreased the strength due to reduction of CH content, alteration of morphology, and formation of micro-cracks as observed from the SEM-EDS analysis.

- CN (1%) at 300<sup>0</sup>C and SSP (7.5% and 10%) at 450<sup>0</sup>C gave satisfactory performance under elevated temperature in comparison to all the mix proportions.
- The microstructure of mortar specimens altered under the exposure to elevated temperatures. As observed from SEM-EDS analysis, at 300<sup>0</sup>C temperature, dense and compact microstructure was formed, C-S-H gel was spread over the matrix, and little amount of ettringite formation was seen in comparison to specimens at room temperature.
- The mixes F3 (7.5% SSP), F4 (10% SSP), F8 (1% CN), F14 (2% CN + 5% SSP), and F16 (2% CN + 5% SSP) were found to have better performance and can be used when exposing temperature rose to 450<sup>0</sup>C.
- The incorporation of SSP in mortar reduced the EE, ECO<sub>2</sub>, and cost among all the additives. The addition of chemical admixtures hiked the cost of mortar construction.
- The performance evaluation of different mix proportions was also evaluated with reference to plain mix.

The effect of elevated temperature (150<sup>0</sup>C, 300<sup>0</sup>C, 450<sup>0</sup>C, and 600<sup>0</sup>C) on mortar mix consisting of accelerators and SSP was studied for change in mass, strength, and microstructural properties compared with specimens at room temperature. The stone with stone slurry powder and calcium nitrate had better performance at elevated temperature than other mix proportions. CN and SSP in mortar can be used when temperatures rise to 300<sup>0</sup>C and 450<sup>0</sup>C, respectively.

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